FLORIDA-ALABAMA TPO
ESCAMBIA-SANTA ROSA RTMC AND RTMC SUPPORT FACILITY

6575 NORTH W Street, PENSACOLA, FL 32505

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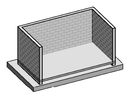

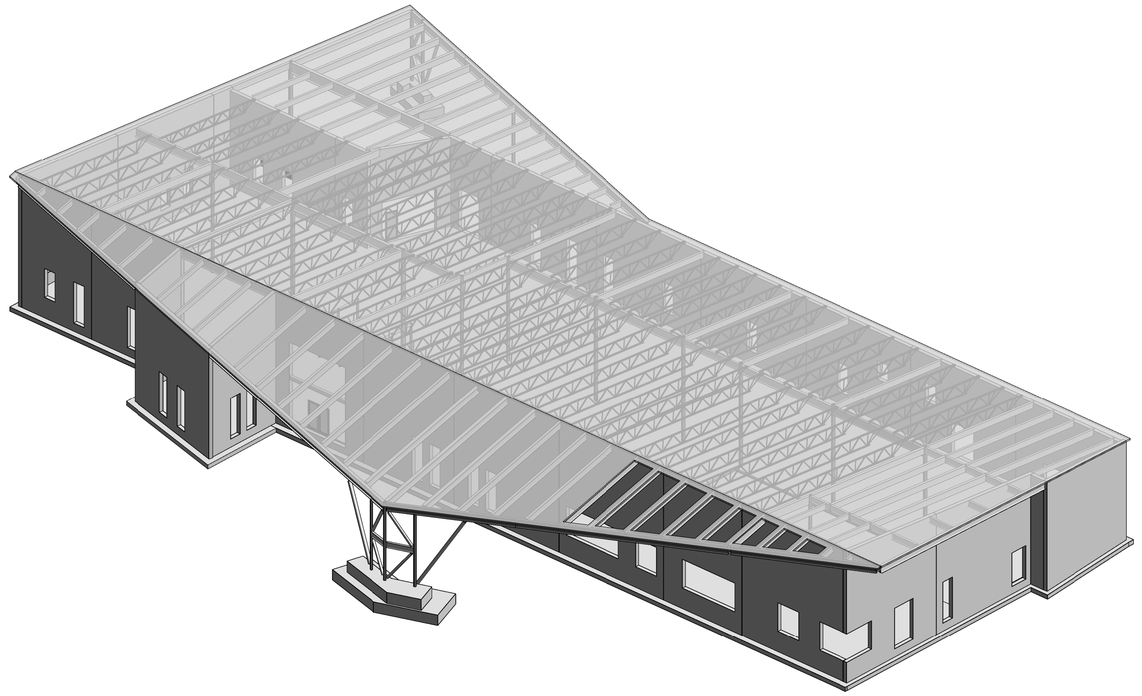

No.	Date	Issue / Revision

CODY LAMBERT, State of Florida, Professional Engineer, License No. 100455

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FLORIDA-ALABAMA TPO		
ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

COVER SHEET

DWG
S-000
SHEET

GENERAL NOTES

DIVISION 01 - GENERAL REQUIREMENTS

1.1 BUILDING CODES & ORDINANCES

A. ALL DESIGN AND CONSTRUCTION SHALL BE IN ACCORDANCE WITH ALL APPLICABLE FEDERAL, STATE, AND LOCAL BUILDING CODES/ORDINANCES AND FIRE CODES, INCLUDING THE FOLLOWING:
   1. FLORIDA BUILDING CODE 2023 (FBC 2023)

1.2 DATUM

A. ALL STRUCTURAL ELEVATIONS ARE RELATIVE ELEVATIONS. REFER TO PLANS FOR RELATIVE 0'-0" DATUM AND CORRESPONDING ACTUAL ELEVATION USING NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD 88).

B. REFER TO PLAN FOR ADDITIONAL INFORMATION

1.3 DEFERRED SUBMITTALS

A. IN ACCORDANCE WITH IBC 107.3.4.1 AND LOCAL BUILDING DEPARTMENT REQUIREMENTS, DEFERRED SUBMITTALS ARE DEFINED AS THOSE PORTIONS OF THE DESIGN THAT ARE NOT SUBMITTED AT THE TIME OF THE APPLICATION FOR PERMIT AND WHICH ARE TO BE SUBMITTED TO THE BUILDING OFFICIAL WITHIN A SPECIFIED PERIOD.

B. DEFERRAL OF ANY SUBMITTAL ITEMS SHALL HAVE PRIOR APPROVAL OF THE BUILDING OFFICIAL. THE ARCHITECT OR ENGINEER OF RECORD SHALL LIST THE DEFERRED SUBMITTALS ON THE PLANS AND CONTRACTOR SHALL SUBMIT THE DEFERRED SUBMITTAL DOCUMENTS FOR REVIEW BY THE BUILDING OFFICIAL.

C. ALL DEFERRED SUBMITTAL ITEMS SHALL BE SIGNED AND SEALED BY A DESIGN PROFESSIONAL LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED.

D. DOCUMENTS FOR DEFERRED SUBMITTAL SHALL BE SUBMITTED TO THE ARCHITECT OR ENGINEER OF RECORD A MINIMUM OF 30 DAYS PRIOR TO FABRICATION. DEFERRED SUBMITTAL DOCUMENTS WILL BE REVIEWED BY THE ARCHITECT OR ENGINEER OF RECORD FOR GENERAL CONFORMANCE WITH CONTRACT DOCUMENTS. A COPY OF THE DEFERRED SUBMITTAL DOCUMENTS SHALL BE SUBMITTED TO THE BUILDING OFFICIAL WITH A NOTATION INDICATING THAT THE DEFERRED SUBMITTAL DOCUMENTS HAVE BEEN REVIEWED AND FOUND TO BE IN GENERAL CONFORMANCE TO THE DESIGN OF THE BUILDING. THE DEFERRED SUBMITTAL ITEMS SHALL NOT BE INSTALLED UNTIL THEIR DESIGN AND SUBMITTAL DOCUMENTS HAVE BEEN APPROVED BY THE BUILDING OFFICIAL.

E. THE ENGINEER OF RECORD IS DELEGATING THE DESIGN OF THE FOLLOWING ITEMS, ALL OF WHICH WILL BE DEFERRED SUBMITTALS:

1 TEMPORARY EARTH RETENTION SYSTEMS
2 GLAZING SYSTEMS AND THEIR CONNECTIONS
3 PRECAST CONCRETE DRAWINGS AND CALCULATIONS (INCLUDING CAST-IN FABRICATED HARDWARE)
4 GUARDRAILS AND HANDRAILS
5 STRUCTURAL STEEL CONNECTION DRAWINGS AND CALCULATIONS (AS SPECIFIED)
6 TEMPORARY BRACING AND SHORING SYSTEMS FOR SLABS, BEAMS, AND WALLS
7 LIGHT GAUGE STEEL FRAMING AND THEIR CONNECTIONS
8 AWNINGS AND OTHER PREFABRICATED/PRE-ENGINEERED SYSTEMS
9 STEEL JOIST FRAMING AND THEIR CONNECTIONS

1.6 LOADING AND DESIGN PARAMETERS - ASCE7-22 OR AS SPECIFIED HEREIN

A.	RISK CATEGORY	IV	
B.	SUPERIMPOSED DEAD LOADING:		
1	ROOF (NON-PARKING):	15	PSF
C.	LIVE LOADING:		
2	ROOF (NON-PARKING):		
i.	UNIFORM LOAD:	20	PSF
ii.	CONCENTRATED LOAD ACTING ON 4 SQ. IN. AREA:	300	LBS
3	SLAB ON GRADE:		
i.	UNIFORM LOAD:	100	PSF
ii.	CONCENTRATED LOAD ACTING ON 20 SQ. IN. AREA:	3000	LBS
4	CONSTRUCTION LIVE LOAD:		
i.	UNIFORM LOAD:	20	PSF
ii.	CONCENTRATED LOAD ACTING ON 20 SQ. IN. AREA:	2000	LBS

D. RAIN LOADING:
1. STRUCTURE IS DESIGNED IN ACCORDANCE WITH ASCE7 FOR EACH PORTION OF ROOF TO SUSTAIN THE LOAD OF RAINWATER THAT WILL ACCUMULATE ON IT IF THE PRIMARY DRAINAGE SYSTEM FOR THAT PORTION IS BLOCKED, INCLUSIVE OF UNIFORM LOAD CAUSED BY WATER THAT RISES ABOVE THE INLET OF THE SECONDARY DRAINAGE SYSTEM AT ITS DESIGN FLOW.

2. DESIGN RAINFALL INTENSITY (15 MINUTE), i — 10.8 PSF

E. ATMOSPHERIC ICE LOADING:
1. STRUCTURE IS NOT DESIGNED FOR ATMOSPHERIC ICING

F. FLOOD LOADING:
1. FLOOD ZONE — ZONE "X"
2. PER FBC 2023 SECTION 1612.1 AND BASED ON ITS FLOOD ZONE DESIGNATION OF ZONE "X", THE BUILDING STRUCTURE NEED NOT BE DESIGNED AND CONSTRUCTED TO RESIST THE EFFECTS OF FLOOD HAZARDS AND FLOOD LOADS .

G. EARTH LATERAL LOADING:
1. SEE ITEM 31.2.A.1

H. WIND LOADING:

1.	ULTIMATE DESIGN WIND SPEED (Vult)	178 MPH
2.	TORNADO SPEED, Vt	73 MPH
3.	NOMINAL DESIGN WIND SPEED (Vasd)	138 MPH
4.	EXPOSURE CATEGORY	C
5.	EFFECTIVE PLAN AREA, Ae, FOR TORNADO DESIGN	40,000 sf
6.	INTERNAL PRESSURE COEFFICIENT (GCpi)	+/-0.18

7. THE FINAL NET DESIGN WIND PRESSURE FOR MAIN WIND FORCE RESISTING SYSTEM DESIGN, INCLUDING ALL PERMITTED REDUCTIONS, SHALL NOT BE LESS THAN 8 PSF FOR THE ROOF AND 16 PSF FOR THE WALLS, PROJECTED ONTO A VERTICAL PLANE NORMAL TO THE ASSUMED WIND DIRECTION.

8. THE FINAL NET DESIGN WIND PRESSURE FOR COMPONENTS AND CLADDING DESIGN, INCLUDING ALL PERMITTED REDUCTIONS, SHALL NOT BE LESS THAN 16 PSF ACTING IN ANY DIRECTION.

9. COMPONENTS AND CLADDING DESIGN PRESSURES (PSF)
SEE WIND PRESSURE DIAGRAMS S-007

10. MAIN WIND FORCE RESISTING SYSTEM (MWFRS) BEHAVIOR:
i. NORTH/SOUTH DIRECTION:
ANALYSIS PROCEDURE:
DIRECTIONAL PROCEDURE (CHAPTER 27)
LATERAL FORCE RESISTING SYSTEM:
ORDINARY PRECAST SHEAR WALLS (A.7 - BEARING WALL SYSTEMS)
ii. EAST/WEST DIRECTION:
ANALYSIS PROCEDURE:
DIRECTIONAL PROCEDURE (CHAPTER 27)
LATERAL FORCE RESISTING SYSTEM:
ORDINARY PRECAST SHEAR WALLS (A.7 - BEARING WALL SYSTEMS)

11. PER ASCE 7-22 SECTION 32.5.2 DESIGN FOR TORNADO LOADS IS NOT REQUIRED.

I. CONCRETE EXPOSURE ZONE — F0 & F1 (ZONE I)

1.12 CONSTRUCTION AND COORDINATION NOTES

A. THE CONTRACTOR SHALL COORDINATE AND CHECK ALL DIMENSIONS RELATING TO ARCHITECTURAL FINISHES, MECHANICAL EQUIPMENT AND OPENINGS, ELEVATOR SHAFTS AND OVERRIDES, ETC. AND NOTIFY THE ARCHITECT/ENGINEER OF ANY DISCREPANCIES BEFORE PROCEEDING WITH ANY WORK.

B. METHODS, PROCEDURES, AND SEQUENCES OF CONSTRUCTION ARE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR OR SUBCONTRACTOR. THE GENERAL CONTRACTOR OR SUBCONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTIONS TO MAINTAIN AND ENSURE THE INTEGRITY OF THE STRUCTURE AT ALL STAGES OF CONSTRUCTION.

C. CONSTRUCTION MEANS, METHODS, PROCEDURES, BRACING, AND SAFETY ARE THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR OR SUBCONTRACTOR. THE STRUCTURAL DRAWINGS REPRESENT THE COMPLETE STRUCTURAL SYSTEM IN ITS FINISHED STATE.

D. THE STRUCTURE HAS BEEN DESIGNED FOR THE IN-SERVICE LOADS ONLY. ALL STRUCTURAL ELEMENTS OF THE PROJECT HAVE BEEN DESIGNED BY THE ENGINEER TO RESIST THE REQUIRED CODE VERTICAL AND LATERAL FORCES THAT COULD OCCUR IN THE FINAL COMPLETED STRUCTURE ONLY. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO PROVIDE ALL REQUIRED BRACING DURING CONSTRUCTION TO MAINTAIN THE STABILITY AND SAFETY OF ALL STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PROCESS UNTIL THE LATERAL-LOAD RESISTING OR STABILITY-PROVIDING SYSTEM IS COMPLETELY INSTALLED AND THE STRUCTURE IS COMPLETELY TIED TOGETHER. TEMPORARY SUPPORTS SHALL NOT RESULT IN THE OVERSTRESS OR DAMAGE OF THE ELEMENTS TO BE BRACED NOR ANY ELEMENTS USED AS BRACE SUPPORTS. THE METHODS, PROCEDURES, AND SEQUENCES OF CONSTRUCTION ARE THE RESPONSIBILITY OF THE CONTRACTOR.

E. ALL OMISSIONS OR CONFLICTS AMONG VARIOUS ELEMENTS OF DRAWINGS AND/OR SPECIFICATIONS SHALL BE REPORTED TO ENGINEER BEFORE PROCEEDING WITH ANY WORK. THE MOST STRINGENT RESTRICTIONS AND REQUIREMENTS SHALL GOVERN A CONFLICT UNLESS NOTED OTHERWISE.

F. NO STRUCTURAL MEMBERS SHALL BE PENETRATED OR CUT FOR PIPES, DUCTS, ETC., UNLESS SPECIFICALLY DETAILED OR APPROVED IN WRITING BY ENGINEER. ALL PENETRATIONS SHALL BE SUBMITTED TO ENGINEER OF RECORD FOR REVIEW AND APPROVAL.

G. VERIFY AND COORDINATE LOCATION AND SIZE OF ALL OPENINGS AND SLEEVES THROUGH FLOORS, WALLS, AND ROOFS WITH RESPECTIVE SUBCONTRACTOR (MECHANICAL, ELECTRICAL, HVAC, ETC.)

H. THE STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE DRAWINGS AND SPECIFICATIONS OF ALL OTHER DISCIPLINES AND COORDINATED WITH THE WORK OF ALL TRADES.

I. VERIFY STRUCTURAL ELEVATIONS AND DIMENSIONS WITH RESPECT TO OTHER TRADES AND DRAWINGS.

J. VERIFY LOCATION OF SLAB DEPRESSIONS, FLOOR DRAINS, INSERTS, AND OTHER RELATED ITEMS.

K. SCALES NOTED ON THE DRAWINGS ARE FOR GENERAL REFERENCE ONLY. NO DIMENSIONAL INFORMATION SHALL BE OBTAINED BY DIRECT SCALING OF THE DRAWINGS.

L. FLOOR SLAB ELEVATIONS INDICATED ON DRAWINGS ARE ESTABLISHED TO PROVIDE POSITIVE DRAINAGE AND PROVIDE REQUIRED HEADROOM CLEARANCE. NOTIFY ENGINEER OF GRADES INDICATED WHICH DO NOT ALLOW FOR MINIMUM HEADROOM CLEARANCE OR POSITIVE DRAINAGE.

M. DO NOT INSTALL CONDUIT IN SUPPORTED SLABS, SLABS-ON-GRADE, COLUMNS, OR WALLS UNLESS EXPLICITLY SHOWN OR NOTED ON STRUCTURAL DRAWINGS OR APPROVED BY ENGINEER OF RECORD.

N. VISITS TO JOB SITE BY ENGINEER TO OBSERVE CONSTRUCTION DO NOT IN ANY WAY MEAN GUARANTEE OF CONTRACTORS WORK NOR RESPONSIBILITY FOR COORDINATION, SUPERVISION, NOR SAFETY AT JOB SITE. PERIODIC SITE OBSERVATION BY FIELD REPRESENTATIVES OF WGI IS SOLELY FOR THE PURPOSE OF DETERMINING IF THE WORK IS PROCEEDING IN ACCORDANCE WITH THE STRUCTURAL CONTRACT DOCUMENTS. THIS LIMITED SITE OBSERVATION IS NOT INTENDED TO BE A CHECK OF THE QUALITY OR QUANTITY OF THE WORK, BUT RATHER A PERIODIC CHECK IN AN EFFORT TO INFORM THE OWNER AGAINST DEFECTS AND DEFICIENCIES IN THE WORK OF THE CONTRACTOR.

O. ONLY LARGER SLEEVE OPENINGS AND FRAMED OPENINGS IN STRUCTURAL FRAMING COMPONENT MEMBERS ARE INDICATED ON THE STRUCTURAL DRAWINGS. HOWEVER, ALL SLEEVES, INSERTS AND OPENINGS, INCLUDING FRAMES AND/OR SLEEVES SHALL BE PROVIDED FOR PASSAGE, PROVISION AND/OR INCORPORATION OF THE WORK OF THE CONTRACT, INCLUDING BUT NOT LIMITED TO MECHANICAL, ELECTRICAL, AND PLUMBING WORK. THIS WORK SHALL INCLUDE THE COORDINATION OF SIZES, ALIGNMENT, DIMENSIONS, POSITION, LOCATIONS, ELEVATIONS, AND GRADES AS REQUIRED TO SERVE THE INTENDED PURPOSE. OPENINGS NOT INDICATED ON THE STRUCTURAL DRAWINGS, BUT REQUIRED AS NOTED ABOVE, SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW.

P. COMPATIBILITY OF THE STRUCTURE AND PROVISIONS FOR BUILDING EQUIPMENT SUPPORTED ON OR FROM STRUCTURAL COMPONENTS SHALL BE VERIFIED AS TO SIZE, DIMENSIONS, CLEARANCES, ACCESSIBILITY, WEIGHTS, AND REACTION WITH THE EQUIPMENT FOR WHICH THE STRUCTURE HAS BEEN DESIGNED PRIOR TO SUBMISSION OF SHOP DRAWINGS AND DATA FOR EACH PIECE OF EQUIPMENT AND FOR STRUCTURAL COMPONENTS. DIFFERENCES SHALL BE NOTED ON THE SUBMITTALS.

Q. THE DETAILS DESIGNATED AS "TYPICAL DETAILS" APPLY GENERALLY TO THE STRUCTURAL DRAWINGS IN ALL AREAS WHERE CONDITIONS ARE SIMILAR TO THOSE DESCRIBED IN THE DETAILS.

R. DO NOT SUSPEND ANY ITEMS, SUCH AS DUCTWORK, MECHANICAL OR ELECTRICAL FIXTURES, CEILINGS, ETC. FROM STEEL ROOF DECK

S. THE MECHANICAL CONTRACTOR SHALL VERIFY THAT MECHANICAL UNITS SUPPORTED BY THE FRAMING ARE CAPABLE OF SPANNING THE DISTANCE BETWEEN THE SUPPORTING MEMBERS INDICATED ON THE STRUCTURAL DRAWINGS. THE MECHANICAL CONTRACTOR SHALL SUPPLY ADDITIONAL SUPPORT FRAMING AS REQUIRED.

T. ALL REQUESTS FOR SUBSTITUTIONS OF MATERIALS OR DETAILS SHOWN IN THE STRUCTURAL CONTRACT DOCUMENTS SHALL BE SUBMITTED FOR APPROVAL DURING THE BIDDING PERIOD.

U. ONCE BIDS ARE ACCEPTED, PROPOSED SUBSTITUTIONS WILL BE CONSIDERED ONLY WHEN THEY ARE OFFICIALLY SUBMITTED WITH AN IDENTIFIED SAVINGS OR DURATION TO BE DEDUCTED FROM THE CONTRACT AND/OR SCHEDULE IMPACT. SUBMITTALS NOT SATISFYING THE ABOVE CRITERIA WILL NOT BE CONSIDERED.

V. STRUCTURAL DRAWINGS ARE NOT PERMITTED TO BE USED AS SHOP DRAWINGS.

W. GC TO ALLOW 10 BUSINESS DAYS MINIMUM PER SHOP DRAWING AND 5 BUSINESS DAYS MINIMUM PER RFI FOR COMPLETE REVIEW. MULTIPLE REVIEWS COMMENCE AT THE DISCRETION OF WGI. U.N.O. EACH SHOP DRAWING WILL BE REVIEWED IN THE ORDER IT WAS RECEIVED. EACH REVIEW WILL BEGIN AFTER RETURNING THE PREVIOUS SHOP DRAWING. WGI WILL NOT ASSUME PRIORITY FOR REVIEW OF MULTIPLE SUBMITTALS RECEIVED. DELAYS DUE TO INADEQUATE PLANNING FOR THIS REQUIRED TIME ARE NOT THE FAULT OF WGI AND WGI SHALL NOT BE HELD LIABLE FOR ANY DELAY CLAIMS.

THE OFFICIAL RECORD OF THIS SHEET IS THE ELECTRONIC FILE DIGITALLY SIGNED AND SEALED UNDER RULE 61G15-23.004, F.A.C.
GENERAL DISCLAIMER: TO THE BEST OF THE ARCHITECT'S OR ENGINEER'S KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE MINIMUM BUILDING CODES AND THE APPLICABLE FIRE SAFETY STANDARDS AS DETERMINED BY THE LOCAL AUTHORITY IN ACCORDANCE WITH FBC 110.8.4.4 AND CHAPTER 63, FLORIDA STATUES.

No.	Date	Issue / Revision

CODY LAMBERT, State of Florida, Professional Engineer, License No. 100455

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FLORIDA-ALABAMA TPO

ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

GENERAL NOTES

DWG S-001 SHEET

DIVISION 03 - CONCRETE

3.1 CAST-IN-PLACE CONCRETE

A. ALL DESIGN AND CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE FOLLOWING:
   1. ACI 318-19

B. CONCRETE MIX DESIGNS:
   1. CONCRETE MIX DESIGNS SHALL BE PER ACI 318 & ACI 362.1R AS MINIMUMS, EXCEPT AS NOTED ON DRAWINGS AND SPECIFIED HEREIN. THE CONCRETE MIX DESIGN SCHEDULE BELOW SHALL GOVERN IN ALL CASES EXCEPT WHERE DEVIATIONS ARE SPECIFICALLY NOTED ON DRAWINGS.

DESCRIPTION	F'C (PSI)	MAX W/C RATIO
FOOTINGS & MAT FDNS	4000 STD	0.45
GRADE BEAMS	4000 STD	0.45
COLUMNS & SHEAR WALLS	4000 STD	0.45
WALLS & PILASTERS	4000 STD	0.45
BEAMS	4000 STD	0.45
SLABS	4000 STD	0.45
TOPPING SLAB	4000 STD	0.45
SLAB-ON-GRADE	4000 STD	0.45
COMPOSITE METAL DECK	4000 STD	0.45
ALL OTHERS	4000 STD	0.45

NOMINAL MAX. AGGREGATE SIZE (IN)	AVG AIR ENTRAINED
3/8	6
1/2	5.5
3/4	5

NOTES:
   i. STD: DESIGNATES A CONCRETE MIX DESIGN IN ACCORDANCE WITH APPLICABLE SPECIFICATION SECTION WHICH DOES NOT REQUIRE SILICA FUME, GGBS, FLY ASH, OR CALCIUM NITRITE ADMIXTURE

   ii. ALL NORMALWEIGHT CONCRETE SHALL HAVE A DENSITY OF APPROXIMATELY 145 PCF UNLESS NOTED OTHERWISE. THE WEIGHT OF SILICA FUME AND GGBS AND FLY ASH ADMIXTURE(S) MAY BE INCLUDED WITH THE WEIGHT OF CEMENT.

   iii. FOR SITEWORK CONCRETE REFER TO CIVIL DRAWINGS AND SPECIFICATIONS

   2. CEMENT:
      i. MATERIAL PROPERTIES:
         a. ASTM C150 TYPE I OR IA
   3. AGGREGATES:
      i. ASTM C33
   4. GROUT:
      i. PREMIXED, PACKAGED, NON-SHRINK, NON-STAINING, AND CHLORIDE-FREE
      ii. MATERIAL PROPERTIES:
         a. ASTM C1107
         b. COMPRESSIVE STRENGTH: 6000 PSI MINIMUM

C. CONCRETE PUMPING, PLACING, FINISHING, AND JOINTING:
   1. GENERAL - PLACING:
      i. THE USE OF CHLORIDES SUCH AS DEICING SALTS ARE PROHIBITED FOR USE OF MELTING ICE PRIOR TO PLACEMENT OF CONCRETE.
   2. GENERAL - FINISHING:
      i. PROVIDE A 3/4 INCH CHAMFER ON EXPOSED CORNERS OF CONCRETE UNLESS NOTED OTHERWISE. TOP EDGES OF WALLS MAY BE TOOLED.
   3. GENERAL - JOINTING:
      i. CONSTRUCTION JOINTS SHALL HAVE CONTACT SURFACES CLEAN AND FREE OF LAITANCE. CONTRUCTION JOINTS SHALL BE PREPARED AS SPECIFIED ON DRAWINGS WITH A SMOOTH INTERFACE, A KEYED INTERFACE, OR A ROUGHENED INTERFACE BY MEANS OF ROUGHENED CONCRETE FINISH OR MECHANICAL ROUGHENING OF THE CONTACT SURFACE TO A FULL AMPLITUDE OF 1/4".

      ii. CONSTRUCTION JOINTS AND POUR SEQUENCING FOR ELEVATED SLABS SHALL BE AS SPECIFIED ON DRAWINGS. TOOL SLAB JOINTS AT THE TIME OF FINISHING.

      iii. JOINTS FOR SLAB-ON-GRADE SHALL BE AS SPECIFIED ON DRAWINGS. SLAB SHALL BE CAST IN STRIPS 15 FEET BY 100 FEET MAXIMUM UNO OR APPROVED BY ENGINEER. TOOL SLAB JOINTS AT THE TIME OF FINISHING OR SAW CUT AS SPECIFIED ON DRAWINGS.

      iv. JOINTS FOR WALLS SHALL BE 15 FEET ON CENTER MAXIMUM UNO

      v. CONSTRUCTION JOINTS IN CONCRETE ELEMENTS WHICH HAVE ONE SIDE EXPOSED TO WEATHER OR EARTH AND THE OTHER SIDE ADJACENT TO ENCLOSED SPACE SHALL BE PROVIDED WITH WATERSTOPS UNLESS NOTED OTHERWISE. REFER TO DRAWINGS FOR ADDITIONAL JOINT WATERPROOFING AND DAMPPROOFING DETAILS.

D. MILD REINFORCEMENT:
   1. DEFORMED BARS:
      i. MATERIAL PROPERTIES:
         a. ASTM A615 GRADE 60
         b. ASTM A706 GRADE 60 (LOW-ALLOY WELDABLE)
      ii. COATING PROPERTIES (AS REQD, SEE ITEM 3.1.D.6):
         a. NO COATING
      iii. HEADED DEFORMED BARS (AS REQD):
         a. ASTM A970
   2. WELDED DEFORMED BAR MATS:
      i. MATERIAL PROPERTIES:
         a. SEE ITEM 3.1.D.1.i
      ii. FABRICATION PROPERTIES:
         a. ASTM A184
   3. WELDED WIRE REINFORCEMENT (PLAIN & DEFORMED):
      i. MATERIAL PROPERTIES:
         a. ASTM A1064 GRADE 70 (DEFORMED)
      ii. FABRICATION PROPERTIES:
         a. ASTM A1064
   4. HEADED SHEAR STUD REINFORCEMENT:
      i. SHEAR STUDS:
         a. MATERIAL PROPERTIES:
            1. YIELD STRENGTH: 51,000 PSI MINIMUM
            2. TENSILE STRENGTH: 65,000 PSI MINIMUM
            3. ELONGATION: 20 PERCENT IN 2 INCHES
            4. REDUCTION OF AREA: 50 PERCENT MINIMUM
            5. TYPE 1 (SINGLE-HEADED) STUDS:
               I. ASTM A29 GRADES 1010-1020
            6. TYPE 2 (DOUBLE-HEADED) STUDS:
               I. ASTM A29 GRADES 1010-1020
               II. ASTM A615 GRADE 60
               III. ASTM A706 GRADE 60 (LOW-ALLOY WELDABLE)
         b. DIMENSIONS:
            1. ASTM A1044
               I. TABLE 3 FOR TYPE 1 (SINGLE-HEADED) STUDS
               II. TABLE 4 FOR TYPE 2 (DOUBLE-HEADED) STUDS
            2. AS SPECIFIED ON DRAWINGS

   5. THE FOLLOWING TYPES OF MILD REINFORCEMENT ARE NOT PERMITTED UNO:
      i. PLAIN BARS
      ii. PLAIN WIRE (DOES NOT INCLUDE WELDED WIRE REINFORCEMENT)
      iii. DEFORMED WIRE (DOES NOT INCLUDE WELDED WIRE REINFORCEMENT)
   6. REINFORCEMENT PROTECTION SHALL BE PER ACI 318 & ACI 362.1R AS MINIMUMS, EXCEPT AS NOTED ON DRAWINGS AND SPECIFIED HEREIN. THE REINFORCEMENT PROTECTION SCHEDULE BELOW SHALL GOVERN IN ALL CASES EXCEPT WHERE DEVIATIONS ARE SPECIFICALLY NOTED ON DRAWINGS. REINFORCEMENT COATINGS FOR DOWEL & SPLICE REINFORCEMENT, WHERE PERMITTED, SHALL BE THE SAME AS SPECIFIED FOR THE PRIMARY REINFORCEMENT.

ELEMENT DESCRIPTION	COVER	COATING REQS
FOOTINGS	3"	UNCOATED
GRADE BEAMS	3"	UNCOATED
WALLS		
FORMED		
NOT EXPOSED TO EARTH OR WEATHER	1 1/2"	UNCOATED
EXPOSED TO EARTH OR WEATHER	2"	UNCOATED
NOT FORMED		
CAST AGAINST EARTH	3"	UNCOATED
COLUMNS		
NOT EXPOSED TO EARTH OR WEATHER	1 1/2"	UNCOATED
EXPOSED TO EARTH OR WEATHER	2"	UNCOATED
PILASTERS	2"	UNCOATED
BEAMS (ELEVATED)	2" TOP 1 1/2" SIDE & BOT	UNCOATED
SLABS (ELEVATED)	2" TOP 1 1/2" SIDE & BOT	UNCOATED
TOPPING SLAB	1 1/2"	UNCOATED
SLAB-ON-GRADE	2" TOP	UNCOATED
STAIRS	1 1/2"	UNCOATED
COMPOSITE METAL DECK	1" TOP	UNCOATED

   7. GENERAL - MILD REINFORCEMENT:
      i. THE FOLLOWING MODIFICATIONS TO REINFORCEMENT ARE PROHIBITED UNLESS NOTED OTHERWISE OR APPROVED BY ENGINEER OF RECORD.
         a. FIELD CUTTING, HEATING, BENDING, OR STRAIGHTENING
         b. WELDING
            1. WELDING SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS SET FORTH IN AWS D1.4 WHEN SPECIFICALLY NOTED OR APPROVED FOR WELDING UNCOATED REINFORCEMENT.

      ii. WHERE LAP SPLICES ARE PERMITTED, THE MINIMUM SPLICE LENGTH SHALL BE BASED ON THE CLASS B TENSION LAP PER ACI 318 UNLESS NOTED OTHERWISE

      iii. WHERE STANDARD HOOKS ARE SPECIFIED, PROVIDE 90 DEGREE HOOKS PER ACI 318 SECTION 25.3 UNLESS NOTED OTHERWISE.

      iv. MECHANICAL SPLICES AND TERMINATIONS AS INDICATED ON DRAWINGS

      v. PROVIDE ADDITIONAL TWO #5 REINFORCEMENT BARS ON ALL FOUR SIDES OF ALL OPENINGS UNO. BARS TO EXTEND TWO FEET BEYOND CORNERS OF OPENING.

E. CONCRETE ACCESSORIES:
   1. SEE DIVISION 05 FOR ALL METAL FABRICATION ACCESSORIES ASSOCIATED WITH CAST-IN-PLACE CONCRETE. THIS SHALL INCLUDE BUT NOT BE LIMITED TO THE FOLLOWING:
      i. EMBEDDED STEEL ELEMENTS AND THEIR ANCHORS:
         a. EMBED PLATES
         b. HEADED STUD ANCHORS (FOR EMBEDS)
         c. DEFORMED BAR ANCHORS (FOR EMBEDS)
      ii. ANCHOR RODS AND ASSOCIATED HARDWARE
      iii. SHEAR CONNECTORS (HEADED STUD ANCHORS) FOR COMPOSITE BEAM BEHAVIOR

F. GENERAL - CAST-IN-PLACE CONCRETE:
   1. NO ALUMINUM CONDUIT OR PRODUCTS CONTAINING ALUMINUM OR ANY OTHER MATERIAL INJURIOUS TO THE CONCRETE SHALL BE EMBEDDED IN CONCRETE.

3.3 TILT-UP CONCRETE WALL PANELS

A. CONCRETE STRENGTH AT LIFTING SHALL BE AS DETERMINED BY LIFTING ANALYSIS, BUT SHALL NOT BE LESS THAN 75 PERCENT OF THE SPECIFIED 28 DAY STRENGTH.

B. WALL PANEL ELEVATION SHOWN IN THE STRUCTURAL DRAWINGS ARE AS VIEWED FROM THE BUILDING EXTERIOR

C. UNLESS NOTED OTHERWISE ON THE STRUCTURAL DRAWINGS, REINFORCE THE WALL PANELS AS FOLLOWS:

   1. PROVIDE 4-#5 EACH WAY IN CENTER OF PANEL.
   2. PROVIDE 1-#5 CONTINUOUS AT TOP, BOTTOM, AND SIDES OF ALL PANELS.
   3. PROVIDE 1-#5 CONTINUOUS AT HEAD, SILL, AND JAMBS OF ALL OPENINGS. EXTEND 2'-6" PAST FACE OF OPENING.
   4. PROVIDE 1-#5 X 4'-0" EACH FACE PLACED DIAGONALLY AT THE CORNERS OF ALL OPENINGS.
   5. PROVIDE 4-#4 3'-0" EACH WAY OVER EACH LIFTING INSERT, MINIMUM.

D. INSERTS, BRACES, AND OTHER ACCESSORIES REQUIRED TO LIFT AND ERECT THE WALL PANELS SHALL BE DESIGNED AND PROVIDED BY THE CONTRACTOR. LIFTING ARRANGEMENT SHALL BE SO DEVISED AS TO PREVENT CRACKING OF THE CONCRETE DUE TO ERECTION STRESSES INCLUDING A 50 PERCENT INCREASE IN FORCES DUE TO IMPACT. CONTRACTOR SHALL PROVIDE ANY ADDITIONAL PANEL REINFORCING STEEL REQUIRED FOR LIFTING AND BRACING STRESSES.

E. HOT DIP GALVANIZE ALL ITEMS REQUIRED FOR WALL PANEL CONNECTIONS INCLUDING EMBEDDED ITEMS IN PANELS AND FOUNDATIONS. ALL GALVANIZED SURFACES AFFECTED BY WELDING SHALL BE TOUCHED UP WITH A COLD GALVANIZING COMPOUND.

F. WELDING OF EMBEDDED ITEMS SHALL BE EXECUTED IN SUCH A MANNER TO PREVENT CRACKING, OR SPALLING OF CONCRETE.

G. PROVIDE AND COORDINATE ALL CAST-IN-PLACE ELEMENTS SUCH AS FINISHES, REGLETS, REVEALS, RUSTICATIONS, CHAMFERS, SLEEVES, PLATES, CONDUITS, OPENINGS AND OTHER ACCESSORIES, AS REQUIRED WITH THE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS.

H. BRACE WALL PANELS UNTIL PERMANENT CONNECTIONS OF FLOOR AND ROOF HAVE BEEN CONNECTED. ROOF DECK SHALL BE COMPLETELY INSTALLED BEFORE BRACING IS REMOVED.

I. WALL PANELS RECEIVING BACKFILL WITH A DIFFERENCE IN ELEVATION OF MORE THAN 3'-0" FROM ONE FACE OF THE PANEL TO THE OTHER SHALL BE BRACED TO RESIST THE UNBALANCED LATERAL PRESSURES UNTIL THE BRACING FLOOR SLAB HAS BEEN PLACED AND HAS ATTAINED ITS 28 DAY STRENGTH.

J. REFER TO THE ARCHITECTURAL DRAWINGS FOR LOCATIONS AND DETAILS OF REVEALS (3/4" MAXIMUM DEPTH) IN EXPOSED WALLS.

K. ALL EMBEDDED PLATES SHALL BE PLACED PRIOR TO CONCRETE PLACEMENT.

REINFORCING STEEL SPLICE NOTES:

ALL REINFORCING STEEL SHALL BE SPLICED AS NOTED BELOW AND AS REQUIRED IN THE ACI BUILDING CODE (LATEST EDITION).

A. Ld NOTED IN THE DETAILS AND TABLES THIS SHEET IS THE STRAIGHT BAR DEVELOPMENT LENGTH PER ACI-318.

B. CLASS B LAP SPLICE TABLE SHALL BE USED FOR ALL LAP SPLICES AND BAR DEVELOPMENT UNLESS NOTED OTHERWISE.

C. ELEVATED SLAB STEEL MARKED "CONTINUOUS" SHALL BE LAPPED WITH A CLASS B LAP SPLICE AND AROUND CORNERS OR INTERSECTIONS WITH A STANDARD 90 DEGREE HOOK.

D. SPLICE ALL VERTICAL BARS IN COLUMNS AND VERTICAL AND HORIZONTAL BARS IN SHEAR WALLS WITH A CLASS B LAP SPLICE UNLESS NOTED OTHERWISE.

E. LAP SPLICES FOR #14 AND LARGER BARS SHALL BE MADE WITH MECHANICAL COUPLERS TO DEVELOP 125% OF THE BARS CAPACITY.

F. INCREASE DEVELOPMENT LENGTH SHOWN IN TABLES BELOW BY 1.25 FOR 75 KSI STEEL.

G. INCREASE DEVELOPMENT LENGTH SHOWN IN TABLES BELOW BY 1.50 FOR EPOXY COATED BARS.

H. INCREASE DEVELOPMENT LENGTH SHOWN IN TABLES BELOW BY 1.30 IF BAR IS TO BE USED AS A TOP BAR IN A BEAM OR SLAB WITH 12" OF FRESH CONCRETE BELOW THE BAR.

I. THE FOLLOWING TABLES ASSUME ONE OF THE BELOW CONDITIONS, PER ACI, ARE MET:
   a. CLEAR SPACING OF BARS BEING DEVELOPED OR SPLICED IS NOT LESS THAN db, CLEAR COVER NOT LESS THAN db AND STIRRUPS OR TIES THROUGHOUT Ld NOT LESS THAN CODE MINIMUM
   b. CLEAR SPACING OF BARS BEING DEVELOPED OR SPLICED NOT LESS THAN 2db AND CLEAR COVER NOT LESS THAN db

CLASS "B" LAP SPLICE SCHEDULE (1.3 Ld)

REINF. SIZE	CONCRETE STRENGTH (PSI)							
	3000	4000	5000	6000	7000	8000	10000	12000
#3	23"	20"	17"	16"	16"	16"	16"	16"
#4	29"	25"	23"	21"	20"	19"	16"	16"
#5	37"	32"	29"	26"	24"	23"	16"	16"
#6	43"	38"	34"	32"	29"	28"	20"	20"
#7	63"	55"	50"	45"	42"	39"	36"	36"
#8	72"	63"	56"	51"	47"	45"	39"	39"
#9	81"	71"	63"	58"	54"	50"	45"	45"
#10	91"	80"	71"	65"	60"	56"	51"	51"
#11	102"	88"	78"	72"	67"	63"	56"	56"

HOOKED REINFORCING STEEL SPLICE NOTES:

ALL HOOKED REINFORCING STEEL SHALL BE AS NOTED BELOW AND AS REQUIRED IN THE ACI BUILDING CODE LATEST EDITION.

A. Lhb NOTED IN THE SCHEDULE BELOW IS THE BASIC TENSION DEVELOPMENT LENGTH FOR STANDARD ACI HOOKS, MEASURED FROM THE CRITICAL SECTION TO THE END OF THE HOOK.

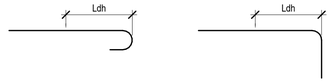
B. Ldh = 1.25*Lhb FOR GRADE 75 (75,000 PSI YIELD) REINFORCEMENT.

C. Ldh = 1.2*Lhb FOR EPOXY COATED REINFORCEMENT.

D. Ldh = Lhb UNLESS CONDITIONS NOTED IN B. OR C. ARE MET AND SHALL BE NOT BE LESS THAT 6" OR 8 BAR DIAMETERS. WHICHEVER IS LARGER.

STANDARD HOOK DEVELOPMENT LENGTH SCHEDULE (Ldh)

REINF. SIZE	CONCRETE STRENGTH (PSI)							
	3000	4000	5000	6000	7000	8000	10000	12000
#3	9"	8"	7"	6"	6"	6"	6"	6"
#4	11"	10"	9"	8"	8"	7"	6"	6"
#5	14"	12"	11"	10"	9"	9"	8"	8"
#6	17"	15"	13"	12"	11"	11"	9"	9"
#7	20"	17"	15"	14"	13"	12"	11"	11"
#8	22"	19"	17"	16"	15"	14"	12"	12"
#9	25"	22"	20"	18"	17"	16"	14"	14"
#10	28"	25"	22"	20"	19"	18"	16"	16"
#11	31"	27"	24"	22"	21"	19"	17"	17"



No.	Date	Issue / Revision

CODY LAMBERT, State of Florida, Professional Engineer, License No. 100455

This item has been digitally signed and sealed by Digitally signed by Cody Lambert Date: 2025.04.01 09:01:45-05'00' on the date indicated here.

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CODY LAMBERT, PE
FL, 100455

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3111 W. DR. MARTIN LUTHER KING JR. BLVD.
SUITE 375
TAMPA, FL 33607

FLORIDA-ALABAMA TPO

ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

GENERAL NOTES

DWG S-002 SHEET

4.1 CONCRETE UNIT MASONRY

A. ALL DESIGN AND CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE FOLLOWING:
   1. TMS 402-16 / TMS 602-16
   2. REFER TO APPLICABLE SPECIFICATION SECTIONS FOR ADDL INFO
B. CONCRETE MASONRY UNITS:
   1. HOLLOW AND SOLID LOAD-BEARING UNITS:
      a. MATERIAL PROPERTIES:
         1. ASTM C90
         2. NET COMPRESSIVE STRENGTH: 2800 PSI MINIMUM
C. MORTAR:
   1. MATERIAL PROPERTIES:
      a. ASTM C270 TYPE S
      b. COMPRESSIVE STRENGTH: 1800 PSI MINIMUM
D. GROUT:
   1. MATERIAL PROPERTIES:
      a. ASTM C476
      b. COMPRESSIVE STRENGTH: 2000 PSI MINIMUM
E. MILD REINFORCEMENT:
   1. DEFORMED BARS:
      a. MATERIAL PROPERTIES:
         1. AS SPECIFIED UNDER CAST-IN-PLACE CONCRETE ITEM 3.1.D.2.i
      b. COATING PROPERTIES:
         1. UNCOATED
      c. HEADED DEFORMED BARS (AS REQD):
         1. AS SPECIFIED UNDER CAST-IN-PLACE CONCRETE ITEM 3.1.D.2.iii
   2. LADDER-TYPE JOINT REINFORCEMENT:
      a. WIRE SHALL BE 9-GAUGE UNLESS NOTED OTHERWISE
      b. MATERIAL PROPERTIES:
         1. ASTM A82
      c. FABRICATION PROPERTIES:
         1. ASTM A951
         2. CROSS WIRES AT 16 INCHES OC
      d. COATING PROPERTIES:
         1. ASTM A153 CLASS B (HOT-DIP GALVANIZED)
   3. THE FOLLOWING TYPES OF MILD REINFORCEMENT ARE NOT PERMITTED UNO:
      a. PLAIN BARS
      b. WELDED DEFORMED BAR MATS
      c. PLAIN WIRE (DOES NOT INCLUDE WELDED WIRE OR LADDER-TYPE REINFORCEMENT)
      d. DEFORMED WIRE (DOES NOT INCLUDE WELDED WIRE REINFORCEMENT)
      e. WELDED WIRE REINFORCEMENT (PLAIN & DEFORMED)
      f. HEADED SHEAR STUD REINFORCEMENT
   4. GENERAL - MILD REINFORCEMENT:
      a. THE FOLLOWING MODIFICATIONS TO REINFORCEMENT ARE PROHIBITED UNLESS NOTED OTHERWISE OR APPROVED BY ENGINEER OF RECORD.
         1. FIELD CUTTING, HEATING, BENDING, OR STRAIGHTENING
         2. WELDING
            a. WELDING SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS SET FORTH IN AWS D1.4 WHEN SPECIFICALLY NOTED OR APPROVED FOR WELDING UNCOATED REINFORCEMENT,
      b. WHERE LAP SPLICES ARE PERMITTED, THE MINIMUM SPLICE LENGTH SHALL BE AS SPECIFIED ON THE DRAWINGS
      c. WHERE STANDARD HOOKS ARE SPECIFIED, PROVIDE 90 DEGREE HOOKS PER ACI 318 SECTION 25.3 UNLESS NOTED OTHERWISE.
      d. MECHANICAL SPLICES AND TERMINATIONS AS INDICATED ON DRAWINGS
F. POST-INSTALLED MILD REINFORCEMENT:
   1. POST-INSTALLED MILD REINFORCEMENT IS PROHIBITED UNLESS NOTED OTHERWISE OR APPROVED BY ENGINEER OF RECORD.
G. MASONRY ACCESSORIES:
   1. DOVETAIL MASONRY ANCHORS:
      a. ANCHOR SLOTS:
         1. MATERIAL PROPERTIES:
            a. 20-GAUGE MINIMUM STEEL
         2. COATING PROPERTIES:
            a. ASTM A153 CLASS B (HOT-DIP GALVANIZED)
      b. ANCHOR WIRE:
         1. WIRE SHALL BE 3/16 INCH DIAMETER UNLESS NOTED OTHERWISE
         2. MATERIAL PROPERTIES:
            a. ASTM A82
         3. COATING PROPERTIES:
            a. ASTM A153 CLASS B (HOT-DIP GALVANIZED)
      c. ANCHOR TAIL:
         1. MATERIAL PROPERTIES:
            a. 12-GAUGE MINIMUM STEEL
         2. COATING PROPERTIES:
            a. ASTM A153 CLASS B (HOT-DIP GALVANIZED)
   2. CONTROL JOINTS SHALL BE SOLID RUBBER OR PVC STRIPS
   3. BOND BREAKER SHALL BE 15 LB ROOFING FELT
   4. MEMBRANE FLASHING SHALL BE 20 MIL PVC
H. GENERAL - CONCRETE UNIT MASONRY:
   1. MASONRY CONTROL JOINTS SHALL BE SPACED AS FOLLOWS OR AS INDICATED ON DRAWINGS:
      a. 4 FEET MAXIMUM FROM CORNERS
      b. 24 FEET MAXIMUM OC
      c. AT ALL CHANGES IN WALL HEIGHT OR THICKNESS
      d. AT ABUTMENT OF WALLS TO COLUMNS
      e. ALIGN JOINTS WITH JOINTS IN SUPPORTING CONCRETE ELEMENTS WHERE APPLICABLE
   2. MAINTAIN A 1 INCH GAP BETWEEN THE TOP OF ALL NON-BEARING INTERIOR MASONRY WALLS AND THE UNDERSIDE OF STRUCTURE ABOVE.
   3. BOND BEAMS SHALL BE FILLED WITH GROUT AND REINFORCED WITH A MINIMUM OF TWO #4 CONTINUOUS OR AS INDICATED ON DRAWINGS. WHERE BOND BEAMS INTERSECT AT CORNERS AT DIFFERENT ELEVATIONS, RUN EACH BOND BEAM AROUND CORNER FOR TWO BLOCK LENGTHS MINIMUM BEFORE TERMINATING.
   4. LINTELS SHALL HAVE A MINIMUM BEARING OF 6 INCHES.

DIVISION 05 - METALS

5.1 STEEL FRAMING

A. ALL DESIGN AND CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE FOLLOWING:
   1. ANSI/AISC 303-10
   2. ANSI/AISC 360-16
   3. REFER TO APPLICABLE SPECIFICATION SECTIONS FOR ADDL INFO

B. FRAMING SHAPES:
   1. W-SHAPES:
      i. MATERIAL PROPERTIES:
         a. ASTM A992 GRADE 50
      ii. COATING PROPERTIES:
         a. AS REQD, SEE ITEM 5.1.E
   2. CHANNELS, ANGLES, M-SHAPES, S-SHAPES:
      i. MATERIAL PROPERTIES:
         a. ASTM A36
         b. ASTM A588 (WHERE SPECIFICALLY NOTED)
      ii. COATING PROPERTIES:
         a. AS REQD, SEE ITEM 5.1.E
   3. HOLLOW STRUCTURAL SECTIONS (HSS):
      i. MATERIAL PROPERTIES:
         a. ASTM A500 GRADE C
         b. ASTM A1085 GRADE A
      ii. COATING PROPERTIES:
         a. AS REQD, SEE ITEM 5.1.E
   4. PIPES:
      i. MATERIAL PROPERTIES:
         a. ASTM A53 GRADE B TYPE E
         b. ASTM A53 GRADE B TYPE S
      ii. COATING PROPERTIES:
         a. AS REQD, SEE ITEM 5.1.E
   5. PLATES AND BARS:
      i. MATERIAL PROPERTIES:
         a. ASTM A36
      ii. COATING PROPERTIES:
         a. AS REQD, SEE ITEM 5.1.E
   6. STAINLESS STEEL SHEETS, STRIPS, PLATES, AND FLAT BARS:
      i. MATERIAL PROPERTIES:
         a. ASTM A666 TYPE 304
      ii. COATING PROPERTIES:
         a. UNCOATED
   7. STAINLESS STEEL BARS AND SHAPES:
      i. MATERIAL PROPERTIES:
         a. ASTM A276 TYPE 304
      ii. COATING PROPERTIES:
         a. UNCOATED

C. THE METALS PROTECTION SCHEDULE BELOW SHALL GOVERN IN ALL CASES EXCEPT WHERE DEVIATIONS ARE SPECIFICALLY NOTED ON DRAWINGS. ALL METAL COATINGS SHALL BE COORDINATED TO BE COMPATIBLE WITH FINAL PAINTING SYSTEMS.

SPEC #	ELEMENT DESCRIPTION	COATING REQUIREMENTS
055700	EMBED PLATES	HOT-DIP GALVANIZED
055700	BRICK SHELF ANGLES	HOT-DIP GALVANIZED
055700	LINTELS	PAINT & PRIME

D. FRAMING CONNECTIONS:
   1. WELDED CONNECTIONS:
      i. ALL WELDING SHALL BE MADE WITH E70XX LOW HYDROGEN ELECTRODES UNLESS NOTED OTHERWISE AND SHALL CONFORM TO THE LATEST EDITION OF THE AMERICAN WELDING SOCIETY SPECIFICATIONS. ALL WELDS SHALL BE PERFORMED BY AN AWS CERTIFIED WELDER. CERTIFICATION SHALL BE APPROPRIATE FOR WELDING METHOD AND POSTION.
      ii. MINIMUM WELD SIZE IS 1/4 INCH UNLESS NOTED OTHERWISE.
      iii. FOR GALVANIZED CONNECTION HARDWARE REQUIRING FIELD WELDING, REMOVE SLAG, WIRE BRUSH, AND APPLY THREE COATS OF ZRC (COLD GALVANIZING) AFTER WELDING.
   2. BOLTED CONNECTIONS:
      i. BOLTED CONNECTIONS SHALL BE MADE USING ONE OF THE FOLLOWING:
         a. BEARING TYPE CONNECTION WITH THREADS INCLUDED IN SHEAR PLANE (CONDITION N) WITH BOLTS, NUTS, AND WASHERS. TURN OF THE NUT METHOD.
         b. SLIP CRITICAL OR OTHER TYPE CONNECTION AS INDICTED ON DRAWINGS.
      ii. BOLT SIZE SHALL BE 3/4 INCH DIAMETER UNLESS NOTED OTHERWISE.
      iii. MATERIAL PROPERTIES:
         a. ASTM F3125 GRADE A325
         b. ASTM F3125 GRADE F1852
         c. ASTM F3125 GRADE A490
         d. ASTM F3125 GRADE F2280
      iv. COATING PROPERTIES:
         a. UNCOATED
         b. ASTM B695 CLASS 50 (MECHANICALLY DEPOSITED ZINC COATING)
   3. CONNECTIONS NOT SPECIFICALLY DETAILED ON DRAWINGS SHALL BE DESIGNED BY FABRICATOR IN ACCORDANCE WITH AISC SPECIFICATIONS
      i. SHOP CONNECTIONS SHALL BE BOLTED OR WELDED.
      ii. FIELD CONNECTIONS SHALL BE BOLTED WHERE POSSIBLE UNLESS NOTED OTHERWISE.
      iii. USE DOUBLE ANGLE CONNECTIONS WHERE POSSIBLE UNLESS NOTED OTHERWISE.
      iv. WHEN NO REACTIONS ARE INDICATED, DESIGN CONNECTIONS AS A SIMPLE SHEAR CONNECTION TO SUPPORT A REACTION VALUE EQUAL TO ONE HALF THE AISC TABULATED ALLOWABLE UNIFORM LOAD VALUE (6 KIPS MINIMUM)
      v. MEMBERS CALLED OUT AS "CONTINUOUS" SHALL BE FABRICATED AND WELDED TO DEVELOP THE FULL TENSILE CAPACITY OF THE MEMBERS AT JOINTS.

E. FRAMING ACCESSORIES:
   1. SHEAR CONNECTORS (HEADED STUD ANCHORS):
      i. MATERIAL PROPERTIES:
         a. ASTM A108 GRADES 1015-1020, AWS D1.1 TYPE A FOR DIA: 1/4" - 3/8"
         b. ASTM A108 GRADES 1015-1020, AWS D1.1 TYPE B FOR DIA: 1/2" - 1"
      ii. COATING PROPERTIES:
         a. UNCOATED
         b. ASTM B695 CLASS 50 (MECHANICALLY DEPOSITED ZINC COATING)
   2. ANCHOR RODS:
      i. MATERIAL PROPERTIES:
         a. ASTM F1554 GRADE 36
      ii. COATING PROPERTIES:
         a. UNCOATED
         b. ASTM A153 CLASS C (HOT-DIP GALVANIZED)
   3. HEAVY HEX NUTS:
      i. MATERIAL PROPERTIES:
         a. ASTM A563
      ii. COATING PROPERTIES:
         a. MATCH MATING COMPONENTS
   4. WASHERS:
      i. MATERIAL PROPERTIES:
         a. ASTM F436
         b. ASTM F844
      ii. COATING PROPERTIES:
         a. MATCH MATING COMPONENTS
   5. PLATE WASHERS:
      i. MATERIAL PROPERTIES:
         a. SEE ITEM 5.1.B.5.1
      ii. COATING PROPERTIES:
         a. MATCH MATING COMPONENTS
   6. HEADED STUD ANCHORS (FOR EMBEDS):
      i. MATERIAL PROPERTIES:
         a. ASTM A108 GRADES 1015-1020, AWS D1.1 TYPE A FOR DIA: 1/4" - 3/8"
         b. ASTM A108 GRADES 1015-1020, AWS D1.1 TYPE B FOR DIA: 1/2" - 1"
      ii. COATING PROPERTIES:
         a. MATCH MATING EMBED
      iii. CONNECTION TO EMBED:
         a. MANUFACTURER TO PROVIDE AUTOMATIC STUD WELD
   7. DEFORMED BAR ANCHORS (FOR EMBEDS):
      i. MATERIAL PROPERTIES:
         a. ASTM A496, AWS D1.1 TYPE C
         b. ASTM A706 GRADE 60 (LOW-ALLOY WELDABLE)
      ii. COATING PROPERTIES:
         a. MATCH MATING EMBED
      iii. CONNECTION TO EMBED:
         a. MANUFACTURER TO PROVIDE AUTOMATIC STUD WELD
         b. SHOP WELDING SHALL BE PERMITTED FOR ASTM A706 DEFORMED BAR ANCHORS. HOWEVER, E70 ELECTRODES ARE NOT PERMITTED FOR GRADE 60 ANCHORS.
            1. FOR ANCHORS ORIENTED PERPENDICULAR TO ANCHORING PLATE, THE FOLLOWING MINIMUM ALL-AROUND FILLET WELDS SHALL BE PROVIDED USING E80 ELECTRODES:

#3: 1/4"	#4: 1/4"	#5: 5/16"	#6: 3/8"	#7: 7/16"	#8: 1/2"

F. GENERAL - STEEL FRAMING:
   1. NO SPLICES IN COLUMNS WILL BE PERMITTED UNLESS SPECIFICALLY NOTED ON DRAWINGS.
G. FABRICATION
   1 SPLICING OF STRUCTURAL STEEL MEMBERS IS PROHIBITED WITHOUT PRIOR APPROVAL OF THE ENGINEER AS TO LOCATION AND TYPE OF SPLICE TO BE MADE. ANY MEMBER HAVING SPLICE NOT SHOWN AND DETAILED ON SHOP DRAWINGS WILL BE REJECTED.
H. ERECTION
   1 ERECTION TOLERANCES OF ANCHOR BOLTS, EMBEDDED ITEMS, AND ALL STRUCTURAL STEEL UNLESS SPECIFIED OTHERWISE ON THE STRUCTURAL DRAWINGS SHALL CONFORM TO THE AISC CODE OF STANDARD PRACTICE.
   2 FIELD CUTTING OF STRUCTURAL STEEL OR ANY FIELD MODIFICATIONS TO STRUCTURAL STEEL SHALL NOT BE MADE WITHOUT PRIOR APPROVAL OF THE ENGINEER.
   3 CONTRACTOR SHALL PROTECT ANY UNPRIMED STRUCTURAL STEEL FROM DETRIMENTAL EFFECTS OF CORROSION, AS REQUIRED, UNTIL THE STEEL IS ENCLOSED AND PROTECTED BY THE NEW CONSTRUCTION.
   4 HOT DIP GALVANIZE AFTER FABRICATION ALL STRUCTURAL STEEL ITEMS AND CONNECTIONS PERMANENTLY EXPOSED TO THE WEATHER, WHETHER SPECIFIED ON THE STRUCTURAL DRAWINGS OR NOT. SUCH ITEMS INCLUDE, BUT ARE NOT LIMITED TO:
      i. SHELF ANGLES
      ii. PARAPET WALL SUPPORTING MEMBERS
      iii. WINDOW WASHING SUPPORT MEMBERS
      iv. ALL EMBEDDED PLATES IN CONCRETE
      v. BUILDING CLADDING SUPPORT STEEL IN SPACE NOT AIR CONDITIONED AND/OR EXPOSED TO MOISTURE OUTSIDE THE EXTERIOR WATERPROOFING SURFACE IF ANY.
      vi. RAILING EXPOSED TO WEATHER.
      vii. EXAMINE THE ARCHITECTURAL AND STRUCTURAL DRAWINGS FOR OTHER ITEMS REQUIRED TO BE HOT DIPPED GALVANIZED. GALVANIZE ALL NUTS, BOLTS, AND WASHERS USED IN CONNECTION WITH SUCH STEEL. FIELD WELDED CONNECTIONS SHALL HAVE WELDS PROTECTED.
   5 GRIND SMOOTH ALL WELDS VISIBLE TO THE PUBLIC. GRIND OFF ALL WELD SPLATTER.
I. CONTRACTOR SHALL COORDINATE STRUCTURAL STEEL FIREPROOFING REQUIREMENTS. ALL INTERIOR STRUCTURAL STEEL, INCLUDING STEEL JOISTS, SCHEDULED OR INDICATED TO RECEIVE SPRAY APPLIED FIREPROOFING SHALL BE DELIVERED TO THE PROJECT SITE UNPRIMED. STEEL EXPOSED TO CORROSIVE CONDITIONS AFTER INSTALLATION SHALL BE PRIMED WITH A PROTECTIVE COATING WHICH DOES NOT DIMINISH THE BOND BETWEEN THE SPRAY APPLIED FIREPROOFING, AND THE STEEL SUBSTRATE. ANY PRIMER, AND/OR COATING APPLIED TO STRUCTURAL STEEL SHALL BE APPROVED FOR USE IN THE APPLICABLE U.L. FIRE RESISTANCE ASSEMBLY USED ON THE PROJECT.
J. FOR LOOSE LINTELS, MASONRY SHELF ANGLES AND OTHER SUCH ITEMS GENERALLY NOT SHOWN ON THE STRUCTURAL DRAWINGS, REFER TO THE ARCHITECTURAL DRAWINGS.
K. STEEL COLUMNS BELOW GRADE SHALL BE ENCASED IN A MINIMUM OF 4" CONCRETE OR PAINTED WITH 2 COATS OF ASPHALTUM PAINT, UNLESS OTHERWISE SHOWN.
L. STEEL BEAMS NOT SPECIFIED TO RECEIVE CAMBER SHALL BE ERECTED SUCH THAT ANY MINOR CAMBER INDUCED FROM ROLLING OR SHOP ASSEMBLY WILL BE UPWARD IN THE FINAL INSTALLED STATE
M. THE ERECTOR SHALL SHIM BETWEEN PARALLEL ROOF BEAMS AND JOISTS WITH DIFFERENTIAL MILL AND INDUCED CAMBERS FOR LEVEL DECK BEARING.
N. REFER TO THE DIVISION 5 STRUCTURAL STEEL SPECIFICATION OF THE PROJECT MANUAL FOR STRUCTURAL STEEL SURFACE PREPARATIONS AND PRIME PAINTING REQUIREMENTS.
O. PROVIDE CAP PLATES/END PLATES TO CLOSE OFF EXPOSED, OPEN ENDS OF ALL TUBULAR MEMBERS, UNLESS NOTED. SEAL WELD WITH PARTIAL PENETRATION SQUARE GROOVE WELDS FOR WATERTIGHT CONDITION.

5.3 STEEL DECKING

A. REFER TO APPLICABLE SPECIFICATION SECTIONS FOR ADDL INFO
B. STEEL DECKING SHALL COMPLY WITH STEEL DECK INSTITUTE SPECIFICATIONS
C. STEEL DECKING SHALL BE SIZED AS INDICATED ON DRAWINGS
D. STEEL DECKING SHALL BE FABRICATED IN CONTINUOUS PANELS BRIDGING FOUR SUPPORTS (SPANNING OVER THREE CONSECUTIVE SPANS) UNLESS NOTED OTHERWISE
E. STEEL DECKING SHALL BE FASTENED AS INDICATED ON DRAWINGS OR SPECIFICATIONS
F. STEEL ROOF DECKING:
   1. ALL DESIGN AND CONSTRUCTION SHALL BE IN ACCORDANCE WITH ANSI/SDI RD-2022
   2. MATERIAL PROPERTIES:
      i. ASTM A653 GRADE 50 MIN
      ii. ASTM A1008 GRADE 50 MIN
   3. COATING PROPERTIES:
      i. PRIME & PAINT, COORDINATE WITH FINAL PAINT SYSTEM
      ii. ASTM A653 (ZINC)
G. COMPOSITE STEEL FLOOR DECKING:
   1. ALL DESIGN AND CONSTRUCTION SHALL BE IN ACCORDANCE WITH ANSI/SDI C-2022
   2. MATERIAL PROPERTIES:
      i. ASTM A653 GRADE 50 MIN
         a. MAXIMUM DESIGN YIELD STRESS SHALL NOT EXCEED 50KSI NOR Fy
      ii. ASTM A1008 GRADE 50 MIN
         a. MAXIMUM DESIGN YIELD STRESS SHALL NOT EXCEED 50KSI NOR Fy
   3. COATING PROPERTIES:
      i. PRIME & PAINT, COORDINATE WITH FINAL PAINT SYSTEM
      ii. ASTM A653 (ZINC)
H. SHEET STEEL FOR GALVANIZED ROOF DECK AND ACCESSORIES SHALL CONFORM TO ASTM A653, STRUCTURAL QUALITY, WITH A MINIMUM YIELD STRENGTH OF 50 KSI. GALVANIZING SHALL CONFORM TO ASTM A653 WITH A MINIMUM COATING OF G90 AS DEFINED IN A653.
I. SHEET STEEL FOR PRIME PAINTED ROOF DECK AND ACCESSORIES SHALL CONFORM TO ASTM A1008 WITH A MINIMUM YIELD STRENGTH OF 50 KSI.
J. ROOF DECK SHALL BE CONTINUOUS OVER FOUR OR MORE SUPPORTS.
K. PLACE DECK PANELS ON STRUCTURAL SUPPORTS AND ADJUST TO FINAL POSITION WITH ENDS LAPPED 2 INCHES OVER STRUCTURAL SUPPORTS. PROVIDE MINIMUM END BEARING OF 2 INCHES.
L. ROOF DECK CONNECTIONS SHALL CONFORM TO THE REQUIREMENTS OF FACTORY MUTUAL CLASS 1-90 (DATA SHEET 1-28, JANUARY 2012) AT MINIMUM. FOR CONNECTION PURPOSES, THE WIDTH OF THE ROOF CORNERS AND PERIMETER, INCLUDING RIDGES, IS DEFINED AS THE SMALLER OF:
   a. 0.1 TIMES THE BUILDING LESSER PLAN DIMENSION.
   b. 0.4 TIMES THE EAVE HEIGHT
   c. MINIMUM WITH OF 4 FEET.
M. POWER DRIVEN FASTENERS SHALL BE SELECTED BY THE CONTRACTOR FOR THE COMBINATIONS OF DECK GAUGE AND DECK SUPPORT MEMBER THICKNESS. SUBMIT PROPOSED FASTENERS WITH COMPLETE MANUFACTURER'S INFORMATION, INCLUDING DIAPHRAGM SHEAR VALUES FOR THE ENGINEER TO REVIEW.
N. PUDDLE WELDS SHALL BE 5/8" MINIMUM DIAMETER AND SHALL BE MADE THROUGH WELD WASHERS FOR DECKING LIGHTER THAN 22 GAUGE.
O. MECHANICAL, ELECTRICAL AND PLUMBING SYSTEMS SHALL NOT BE SUPPORTED BY THE METAL ROOF DECK.

5.5 STEEL JOISTS

A. REFER TO APPLICABLE SPECIFICATION SECTIONS FOR ADDL INFO
B. STEEL JOISTS SHALL COMPLY WITH STEEL JOIST INSTITUTE SPECIFICATIONS
C. STEEL JOISTS SHALL BE SIZED AS INDICATED ON DRAWINGS
D. K-SERIES, LH-SERIES, DLH-SERIES OPEN WEB STEEL JOISTS AND JOIST GIRDERS:
   1. ALL DESIGN AND CONSTRUCTION SHALL BE IN ACCORDANCE WITH SJI 100-2015
   2. MATERIAL PROPERTIES:
      i. ASTM A36
   3. COATING PROPERTIES:
      i. UNCOATED
      ii. PRIME & PAINT, COORDINATE WITH FINAL PAINT SYSTEM

DIVISION 07 - THERMAL AND MOISTURE PROTECTION

7.1 CONCRETE SEALER

A. CONCRETE SEALER SHALL BE APPLIED TO ALL SLAB ON GRADE AND SUPPORTED SLAB SURFACES AND EXTENDED UP VERTICAL SURFACES 24 INCHES.

7.3 JOINT SEALANTS

A. ALL JOINTS BETWEEN STRUCTURAL (AND ARCHITECTURAL) MEMBERS SHALL BE PROPERLY PREPARED AND FILLED WITH JOINT SEALANT UNLESS NOTED OTHERWISE. ALL JOINT EDGES, INCLUDING TOP AND BOTTOM OF HORIZONTAL SURFACES AND BOTH FACES OF VERTICAL SURFACES, SHALL BE FORMED OR TOOLED AS REQUIRED. JOINT SEALANT SHALL BE APPLIED ONLY TO TOP OF HORIZONTAL SURFACES AND BOTH FACES OF VERTICAL SURFACES UNLESS NOTED OTHERWISE ON DRAWINGS.

B. JOINTS TO BE PREPARED AND FILLED WITH JOINT SEALANT SHALL INCLUDE, BUT ARE NOT LIMITED TO, CONSTRUCTION JOINTS, CONTROL JOINTS, ISOLATION JOINTS, AND ALL INTERFACE JOINTS BETWEEN SIMILAR AND DISSIMILAR MEMBERS. SPECIFIC LOCATIONS MAY BE INDICATED ON DRAWINGS OR MAY BE REQUIRED BY SHOP DRAWINGS OR MAY OCCUR DUE TO CONSTRUCTION SEQUENCE SELECTED BY CONTRACTOR.

No.	Date	Issue / Revision

CODY LAMBERT, State of Florida, Professional Engineer, License No. 100455

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CODY LAMBERT, PE
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3111 W. DR. MARTIN LUTHER KING JR. BLVD.
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TAMPA, FL 33607

FLORIDA-ALABAMA TPO

ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

GENERAL NOTES

DWG
S-003
SHEET

DIVISION 31 - EARTHWORK

31.1 FOUNDATIONS AND OTHER EARTH BEARING SYSTEMS

A. FOUNDATIONS AND OTHER EARTH BEARING SYSTEMS ARE DESIGNED IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE FOLLOWING GEOTECHNICAL INVESTIGATION REPORT:
   1. REPORT NAME: FINAL REPORT OF GEOTECHNICAL INVESTIGATION ESCAMBIA-SANTA ROSA RTMC AND ECRC OFICE FACILITY (NO. 301-35-23-01)
   2. REPORT DATE: MAY 10, 2024
   3. PREPARED BY: ENVIRONMENTAL AND GEOTECHNICAL SPECIALISTS, INC.
   4. REFER TO APPLICABLE SPECIFICATION SECTIONS FOR ADDL INFO

B. OTHER EARTH BEARING SYSTEMS:
   1. CAST-IN-PLACE CONCRETE SLAB-ON-GRADE:
      i. SIZES AND LOCATIONS AS SPECIFIED ON DRAWINGS.
      ii. MATERIAL PROPERTIES AS SPECIFIED IN DIVISION 03.
      iii. CONCRETE ELEMENTS ON NATIVE SOIL OR ENGINEERED FILL:
         a. BEARING STRATUM SHALL COMPLY WITH THE REQUIREMENTS OF THE GEOTECHNICAL INVESTIGATION REPORT.
         b. LOCATION SHALL BE ASSUMED AS TYPICAL UNLESS NOTED OTHERWISE.
         c. SUBGRADE SHALL BE PREPARED IN ACCORDANCE WITH SECTION "RTMC & ECRC BUILDING - STRUCTURAL FILL" OF THE GEOTECHNICAL REPORT
         d. ALL MATERIALS TO BE COMPACTED STANDARD PROCTOR ASTM D698. TESTING LABORATORY TO VERIFY ADEQUACY OF SUBGRADE PREPARATION, FILL MATERIAL AND COMPACTION AS FOLLOWS: 95%
         e. PROVIDE A 15 MIL POLYOLEFIN VAPOR BARRIOR AT GROUND LEVEL. PLACE VAPOR BARRIOR IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS, BUT WITH JOINTS LAPPED NOT LESS THAN 6 INCHES, ON TOP OF STRUCTURAL FILL.
         f. TERMITE PROTECTION MEASURES SHALL BE PROVIDED IN ACCORDANCE WITH SECTION 1816 "TERMITE PROTECTION" OF THE FLORIDA BUILDING CODE

   2. SITE PAVINGS, WALKS, AND AREAS OF GENERAL GRADING/BACKFILLING:
      i. AS SPECIFIED BY CIVIL UNLESS NOTED OTHERWISE
      ii. CONCRETE ELEMENTS ON NATIVE SOIL OR ENGINEERED FILL:
         a. BEARING STRATUM SHALL COMPLY WITH THE REQUIREMENTS OF THE GEOTECHNICAL INVESTIGATION REPORT.
         b. ALL MATERIALS TO BE COMPACTED STANDARD PROCTOR ASTM D698. TESTING LABORATORY TO VERIFY ADEQUACY OF SUBGRADE PREPARATION, FILL MATERIAL AND COMPACTION AS FOLLOWS: 95%

C. SHALLOW FOUNDATIONS:
   1 FOUNDATIONS SHALL BE PLACED AT ELEVATIONS SPECIFIED ON DRAWINGS. A MINIMUM OF 2'-0" SEPERATION SHALL BE PROVIDED BETWEEN CLAYEY SOILS AND THE BOTTOM OF PROPOSED STRUCTURAL FOOTINGS AND FOOTINGS SHALL BEAR AT LEAST 2'-0" BELOW FINISHED GRADE IN ACCORDANCE WITH THE GEOTECHNICAL REPORT.

   2 CAST-IN-PLACE CONCRETE FOOTINGS, MATS, AND STRIPS:
      i. SIZES AND LOCATIONS AS SPECIFIED ON DRAWINGS.
      ii. MATERIAL PROPERTIES AS SPECIFIED IN DIVISION 03.
      iii. CONCRETE ELEMENTS ON NATIVE SOIL OR ENGINEERED FILL:
         a. BEARING STRATUM SHALL COMPLY WITH THE GEOTECHNICAL INVESTIGATION REPORT
         b. LOCATION SHALL BE ASSUMED AS TYPICAL UNLESS NOTED OTHERWISE.
         c. ALLOWABLE DESIGN CAPACITIES:
            1. GRAVITY BEARING (UNIFORM UNO)
               a. COLUMN FOOTING 3950 PSF (NET)
               b. STRIP FOOTING 4000 PSF (NET)
            2. SLIDING COEFFICIENT .36

31.2 PERMANENT LATERAL EARTH RETENTION

A. STRUCTURE IS DESIGNED IN ACCORDANCE WITH ASCE7 FOR EACH PORTION AT GRADE SUBJECTED TO LATERAL EARTH PRESSURES. LATERAL EARTH PRESSURES SPECIFIED HEREIN ARE IN ACCORDANCE WITH THE GEOTECHNICAL INVESTIGATION FOR THE TYPES OF BEHAVIOR GIVEN TO EACH LATERAL EARTH RETAINING ELEMENT.

   1. EARTH LATERAL LOADING:
      i. ACTIVE PRESSURE 38 PSF/FT
      ii. AT-REST PRESSURE 58 PSF/FT
      iii. PASSIVE PRESSURE 173 PSF/FT

31.3 GENERAL - EARTHWORK

A. ALL FOUNDATIONS AND OTHER EARTH BEARING SYSTEMS, PERMANENT LATERAL EARTH RETAINING ELEMENTS, EARTHWORK, EXCAVATION, BACKFILL, AND SUBGRADE DRAINAGE SHALL BE SUBJECT TO OBSERVATION BY GEOTECHNICAL ENGINEER WHOSE APPROVAL IS REQUIRED PRIOR TO PLACEMENT OF OTHER CONSTRUCTION MATERIALS OR CONCRETE. CONTRACTOR SHALL COORDINATE THE CONSTRUCTION SCHEDULE WITH THE GEOTECHNICAL ENGINEER TO FACILITATE OBSERVATION.

B. CONTRACTOR SHALL DETERMINE IN FIELD HORIZONTAL AND VERTICAL LOCATION OF ANY EXISTING UTILITY LINES AND/OR APPURTENANCES AND ADVISE ENGINEER OF ANY CONFLICTS WITH NEW STRUCTURE PRIOR TO CONSTRUCTION. DO NOT DESTROY ANY EXISTING UNDERGROUND STRUCTURES UNLESS AUTHORIZATION IS OBTAINED PRIOR TO CONSTRUCTION.

C. CONTRACTOR SHALL MAINTAIN SAFETY IN CONNECTION WITH EARTH SLOPES CAUSED BY TRENCHING, EXCAVATION, AND/OR FILL DURING CONSTRUCTION. WHERE HEIGHT OF SUCH SLOPES WILL EXCEED 8 FEET OR AS SPECIFIED ON DRAWINGS, SUBMIT FOR RECORD LAYOUT DRAWINGS INDICATING DESIGN OF TEMPORARY EARTH RETENTION SYSTEM WHICH WILL BE IMPLEMENTED (SUCH AS SHEETING AND SHORING OR OTHER METHODS), PREPARED, SIGNED, AND SEALED BY A GEOTECHNICAL AND/OR STRUCTURAL ENGINEER REGISTERED IN STATE WHICH PROJECT IS LOCATED. REFER TO SPECIFICATION SECTION FOR ADDL INFO.

D. ANY UNUSUAL SOIL CONDITIONS (WATER, SOFT LAYERS, ODORS, ETC.) ENCOUNTERED DURING EXCAVATION SHOULD BE IMMEDIATELY BROUGHT TO ATTENTION OF THE GEOTECHNICAL ENGINEER.

No.	Date	Issue / Revision

CODY LAMBERT, State of Florida, Professional Engineer, License No. 100455

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FLORIDA-ALABAMA TPO

ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

GENERAL NOTES

DWG S-004 SHEET

SPECIAL INSPECTIONS

I. GENERAL

A. THE OWNER SHALL EMPLOY ONE OR MORE APPROVED AGENCIES (SPECIAL INSPECTORS) WHO SHALL PROVIDE INSPECTIONS AND MATERIALS TESTING DURING CONSTRUCTION. ALL SPECIAL INSPECTIONS AND TESTING SHALL CONFORM TO THE REQUIREMENTS OF THE IBC SECTION 1705.

B. SPECIAL INSPECTIONS ARE IN ADDITION TO THE INSPECTIONS CONDUCTED BY THE LOCAL BUILDING OFFICIAL AND SHALL NOT RELIEVE THE OWNER AND CONTRACTOR FROM REQUESTING THE BUILDING OFFICIAL'S INSPECTIONS REQUIRED BY IBC SECTION 110.

C. THE SPECIAL INSPECTOR IS OBLIGATED TO BOTH THE OWNER AND THE BUILDING OFFICIAL FOR OBSERVING THAT THE WORK IS EXECUTED IN CONFORMANCE WITH THE CONTRACT DOCUMENTS. THE CONTRACT DOCUMENTS ARE DEFINED AS THE APPROVED, PERMITTED PLANS AND SPECIFICATIONS, AND ALL ADDENDA.

D. THE OWNER SHALL ARRANGE FOR ALL NECESSARY CONSTRUCTION RECORDS TO BE FURNISHED TO THE SPECIAL INSPECTOR IN A TIMELY MANNER. SUCH RECORDS SHALL CONSIST OF, BUT MAY NOT BE LIMITED TO APPROVED CONSTRUCTION DOCUMENTS, APPROVED SHOP DRAWINGS, CONCRETE CYLINDER TEST REPORTS, ETC.

E. DELEGATED DESIGN ITEMS LISTED IN SECTION 1.3 "DEFERRED SUBMITTALS" SHALL HAVE SPECIAL INSPECTIONS THAT ARE DETERMINED BY THE ENGINEER-OF-RECORD FOR THE SPECIFIC DELEGATED DESIGN ITEM.

II. SPECIAL INSPECTOR QUALIFICATIONS

A. THE SPECIAL INSPECTOR SHALL PROVIDE WRITTEN DOCUMENTATION TO THE BUILDING OFFICIAL DEMONSTRATING HIS OR HER COMPETENCE AND RELEVANT EXPERIENCE OR TRAINING. THESE QUALIFICATIONS ARE IN ADDITION TO QUALIFICATIONS SPECIFIED IN OTHER SECTIONS OF THE IBC AND SHALL BE REVIEWED AND ACCEPTED BY THE BUILDING OFFICIAL PRIOR TO THE COMMENCEMENT OF SPECIAL INSPECTIONS WORK.

1 WRITTEN DOCUMENTATION MAY INCLUDE THE CERTIFICATE OF SUCCESSFUL COMPLETION OF AN APPROVED COURSE OR CERTIFICATION AS REQUIRED BY THE JURISDICTION. IT SHALL BE THE SPECIAL INSPECTOR'S RESPONSIBILITY TO BE AWARE OF, AND MAINTAIN CURRENT CERTIFICATION, FOR ANY APPROVED COURSE OR CERTIFICATION REQUIRED BY THE BUILDING OFFICIAL THROUGHOUT THE CONSTRUCTION OF THIS PROJECT.

B. THE SPECIAL INSPECTOR SHALL BE OBJECTIVE, COMPETENT, AND INDEPENDENT FROM THE CONTRACTOR RESPONSIBLE FOR THE WORK BEING INSPECTED. THE SPECIAL INSPECTOR SHALL ALSO DISCLOSE POSSIBLE CONFLICTS OF INTEREST SO THAT OBJECTIVITY CAN BE CONFIRMED.

C. THE SPECIAL INSPECTOR SHALL HAVE ADEQUATE EQUIPMENT TO PERFORM REQUIRED TESTS. THE EQUIPMENT SHALL BE PERIODICALLY CALIBRATED IN ACCORDANCE WITH MANUFACTURER RECOMMENDATIONS OR AS REQUIRED BY CODE OR SPECIFICATION.

D. THE SPECIAL INSPECTOR SHALL EMPLOY EXPERIENCED PERSONNEL EDUCATED IN CONDUCTING, SUPERVISING, AND EVALUATING TESTS AND/OR INSPECTIONS.

E. THE SPECIAL INSPECTOR SHALL POSSESS A THOROUGH KNOWLEDGE OF THE CONTRACT DOCUMENTS AND APPROPRIATE PORTIONS OF THE GOVERNING CODE.

III. REQUIREMENTS OF THE SPECIAL INSPECTOR

A. THE SPECIAL INSPECTOR SHALL MAINTAIN WRITTEN RECORDS OF INSPECTIONS.

B. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE.

C. REQUIREMENTS FOR THESE INSPECTION REPORTS SHALL BE AS FOLLOWS:

1 THE REPORTS SHALL BE IN WRITING.

2 A SEPARATE REPORT SHALL BE SUBMITTED FOR EACH DAY INSPECTIONS AND/OR TESTING WERE PERFORMED.

3 EACH REPORT SHALL CONTAIN THE FOLLOWING INFORMATION.

a. DATE
b. PROJECT NAME
c. PROJECT LOCATION
d. BUILDING PERMIT NUMBER
e. CONTRACTOR'S NAME
f. NAME OF INDIVIDUAL PREPARING THE REPORT
g. NAMES OF KEY INDIVIDUALS WHO WERE PRESENT DURING THE INSPECTION PROCESS
h. IDENTIFICATION OF STRUCTURAL COMPONENTS BEING INSPECTED
i. OBSERVATIONS NOTED DURING INSPECTION
j. ALL DISCREPANCIES AND DEVIATIONS FROM THE CONTRACT DOCUMENTS
k. INDICATION IF THE ITEMS INSPECTED WERE OR WERE NOT IN CONFORMANCE WITH THE CONTRACT DOCUMENTS

D. THE SPECIAL INSPECTOR SHALL MAINTAIN A WRITTEN DISCREPANCY LOG WHERE ANY DISCREPANCIES AND/OR DEVIATIONS FROM THE CONTRACT DOCUMENTS OBSERVED BY THE SPECIAL INSPECTOR ARE RECORDED.

E. DISCREPANCIES AND/OR DEVIATIONS FROM THE CONTRACT DOCUMENTS SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF THESE DISCREPANCIES AND/OR DEVIATIONS ARE NOT CORRECTED, THE DISCREPANCIES/DEVIATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE PRIOR TO THE COMPLETION OF THAT PHASE OF WORK. CORRECTIONS OF ANY DISCREPANCIES SHALL BE RE-INSPECTED BY THE SPECIAL INSPECTOR AND PHOTOGRAPHED FOR RECORD IN THE DISCREPANCY LOG.

F. A FINAL REPORT DOCUMENTING ALL OF THE REQUIRED SPECIAL INSPECTIONS AND THE CORRECTIONS OF ALL OF THE DISCREPANCIES/DEVIATIONS NOTED IN THE INSPECTION REPORTS SHALL BE SUBMITTED TO THE BUILDING OFFICIAL AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE.

IV. REQUIREMENTS OF THE CONTRACTOR

A. ACCESS FOR SPECIAL INSPECTION: THE CONSTRUCTION OR WORK FOR WHICH SPECIAL INSPECTION IS REQUIRED SHALL REMAIN ACCESSIBLE AND EXPOSED FOR SPECIAL INSPECTION PURPOSES UNTIL COMPLETION OF THE REQUIRED SPECIAL INSPECTIONS.

B. THE CONTRACTOR SHALL ADVISE THE SPECIAL INSPECTOR IN ADVANCE OF CONSTRUCTION SCHEDULES AND PLANNED OPERATIONS TO ASSURE TIMELY AND APPROPRIATE INSPECTIONS AND OBSERVATIONS. A MINIMUM OF 24 HOURS' NOTICE SHALL BE GIVEN FOR ALL REQUESTED INSPECTIONS.

C. EACH CONTRACTOR RESPONSIBLE FOR THE CONSTRUCTION OF A MAIN WIND- OR SEISMIC FORCE-RESISTING SYSTEM, DESIGNATED SEISMIC SYSTEM OR A WIND- OR SEISMIC-RESISTING COMPONENT LISTED IN THE STATEMENT OF SPECIAL INSPECTIONS SHALL SUBMIT A WRITTEN STATEMENT OF RESPONSIBILITY TO THE BUILDING OFFICIAL AND THE OWNER PRIOR TO THE COMMENCEMENT OF WORK ON THE SYSTEM OR COMPONENT. THE CONTRACTOR'S STATEMENT OF RESPONSIBILITY SHALL CONTAIN ACKNOWLEDGEMENT OF AWARENESS OF THE SPECIAL INSPECTION REQUIREMENTS CONTAINED IN THE STATEMENT OF SPECIAL INSPECTION.

V. FABRICATORS

A. INSPECTION OF FABRICATORS: WHERE FABRICATION OF STRUCTURAL LOAD-BEARING MEMBERS AND ASSEMBLIES IS PERFORMED ON THE PREMISES OF A FABRICATOR'S SHOP, SPECIAL INSPECTION OF THE FABRICATED ITEMS SHALL BE REQUIRED AND IN ACCORDANCE WITH SECTION 1704.2.5.1 "FABRICATION AND IMPLEMENTATION PROCEDURES" AND AS REQUIRED ELSEWHERE IN THE IBC.

1 EXCEPTION: SPECIAL INSPECTIONS OF FABRICATORS AS REQUIRED BY SECTION 1704.2.5 SHALL NOT BE REQUIRED WHERE THE FABRICATOR IS APPROVED IN ACCORDANCE WITH SECTION 1704.2.5.2.

B. FABRICATOR APPROVAL: SPECIAL INSPECTIONS REQUIRED BY SECTION 1705 ARE NOT REQUIRED WHERE THE WORK IS DONE ON THE PREMISES OF A FABRICATOR REGISTERED AND APPROVED TO PERFORM SUCH WORK WITHOUT SPECIAL INSPECTION. AT COMPLETION OF FABRICATION, THE APPROVED FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE BUILDING OFFICIAL STATING THAT THE WORK WAS PERFORMED IN ACCORDANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS.
FABRICATOR APPROVAL: SPECIAL INSPECTIONS REQUIRED BY SECTION 1705 ARE NOT REQUIRED WHERE THE WORK IS DONE ON THE PREMISES OF A FABRICATOR REGISTERED AND APPROVED TO PERFORM SUCH WORK WITHOUT SPECIAL INSPECTION. AT COMPLETION OF FABRICATION, THE APPROVED FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE BUILDING OFFICIAL STATING THAT THE WORK WAS PERFORMED IN ACCORDANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS.

VI. STATEMENT OF SPECIAL INSPECTION

A. THE FOLLOWING SCHEDULE OF SPECIAL INSPECTIONS (SSI) IS REQUIRED FOR THIS PROJECT.

TABLE 1705.6
REQUIRED VERIFICATION AND INSPECTION OF SOILS

SPECIAL INSPECTION REQUIRED	VERIFICATION AND INSPECTION	CONTINUOUS DURING TASK LISTED	PERIODIC DURING TASK LISTED
YES	1. VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY	-	X
YES	2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL	-	X
YES	3. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS	-	X
YES	4. DURING FILL PLACEMENT, VERIFY USE OF PROPER MATERIALS AND PROCEDURES IN ACCORDANCE WITH THE PROVISIONS OF THE APPROVED GEOTECHNICAL REPORT. VERIFY DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL	X	-
YES	5. PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY	-	X

AISC 360-16 (TABLE N5.6-1)
REQUIRED VERIFICATION AND INSPECTION TASKS FOR BOLTING OF STRUCTURAL STEEL

SPECIAL INSPECTION REQUIRED	VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	REFERENCED STANDARD	IBC REFERENCE
	1. INSPECTION TASKS PRIOR TO BOLTING				
YES	A. MANUFACTURER'S CERTIFICATIONS AVAILABLE FOR FASTENER MATERIALS	X	-	AISC 360-16: N5.6-1	1705.2.1
YES	B. FASTENERS MARKED IN ACCORDANCE WITH ASTM REQUIREMENTS	-	X	AISC 360-16: N5.6-1	1705.2.1
YES	C. CORRECT FASTENERS SELECTED FOR THE JOINT DETAIL (GRADE, TYPE, BOLT LENGTH IF THREADS ARE TO BE EXCLUDED FROM SHEAR PLANE)	-	X	AISC 360-16: N5.6-1	1705.2.1
YES	D. CORRECT BOLTING PROCEDURE SELECTED FOR JOINT DETAIL	-	X	AISC 360-16: N5.6-1	1705.2.1
YES	E. CONNECTING ELEMENTS, INCLUDING THE APPROPRIATE FAYING SURFACE CONDITION AND HOLE PREPARATION, IF SPECIFIED, MEET APPLICABLE REQUIREMENTS	-	X	AISC 360-16: N5.6-1	1705.2.1
YES	F. PRE-INSTALLATION VERIFICATION TESTING BY INSTALLATION PERSONNEL OBSERVED AND DOCUMENTED FOR FASTENER ASSEMBLIES AND METHODS USED	X	-	AISC 360-16: N5.6-1	1705.2.1
YES	G. PROTECTED STORAGE PROVIDED FOR BOLTS, NUTS, WASHERS AND OTHER FASTENER COMPONENTS	-	X	AISC 360-16: N5.6-1	1705.2.1
	2. INSPECTION TASKS DURING BOLTING				
YES	A. FASTENER ASSEMBLIES, OF SUITABLE CONDITION, PLACED IN ALL HOLES AND WASHERS (IF REQUIRED) ARE POSITIONED AS REQUIRED	-	X	AISC 360-16: N5.6-2	1705.2.1
YES	B. JOINT BROUGHT TO THE SNUG-TIGHT CONDITION PRIOR TO THE PRETENSIONING OPERATION	-	X	AISC 360-16: N5.6-2	1705.2.1
YES	C. FASTENER COMPONENT NOT TURNED BY THE WRENCH PREVENTED FROM ROTATING	-	X	AISC 360-16: N5.6-2	1705.2.1
YES	D. FASTENERS ARE PRETENSIONED IN ACCORDANCE WITH THE RCSC SPECIFICATION, PROGRESSING SYSTEMATICALLY FROM THE MOST RIGID POINT TOWARD THE FREE EDGES	-	X	AISC 360-16: N5.6-2	1705.2.1
	3. INSPECTION TASKS AFTER BOLTING				
YES	A. DOCUMENT ACCEPTANCE OR REJECTION OF BOLTED CONNECTIONS	X	-	AISC 360-16: N5.6-3	1705.2.1

TABLE 1705.3
REQUIRED VERIFICATION AND INSPECTION OF CONCRETE CONSTRUCTION

SPECIAL INSPECTION REQUIRED	VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	REFERENCED STANDARD(a)	IBC REFERENCE
YES	1. INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT.	-	X	ACI 318 Ch. 20, 25.2, 25.3, 26.6.1-26.6.3	-
YES	2A. INSPECTION OF REINFORCING BAR WELDING - VERIFICATION OF WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706	-	X	AWS D1.4; ACI 318: 26.6.4	-
YES	2B. INSPECTION OF REINFORCING BAR WELDING - INSPECTION OF SINGLE-PASS FILLET WELDS, MAXIMUM 5/16"	-	X	AWS D1.4; ACI 318: 26.6.4	-
YES	2C. INSPECTION OF ALL OTHER WELDS	-	X	AWS D1.4; ACI 318: 26.6.4	-
YES	3. INSPECTION OF ANCHORS CAST IN CONCRETE	-	X	ACI 318: 17.8.2	-
YES	4A. INSPECTION OF ANCHORS POST-INSTALLED IN HARDENED MEMBERS(b)- ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS.	X	-	ACI 318: 17.8.2.4	-
YES	4B. INSPECTION OF ANCHORS POST-INSTALLED IN HARDENED MEMBERS(b) - MECHANICAL AND ADHESIVE ANCHORS NOT DEFINED IN 4A.	-	X	ACI 318: 17.8.2	-
YES	5. VERIFICATION OF USE OF REQUIRED DESIGN MIX.	-	X	ACI 318: Ch. 19, 26.4.3, 26.4.4	1904.1, 1904.2
YES	6. PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE	X	-	ASTM C 172; ASTM C 31; ACI 318: 26.5, 26.12	-
YES	7. INSPECTION OF CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES	X	-	ACI 318: 26.5	-
YES	8. VERIFICATION OF MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	-	X	ACI 318: 25.5.3-26.5.5	-
NO	9A. INSPECTION OF PRESTRESSED CONCRETE: APPLICATION OF PRESTRESSING FORCES	X	-	ACI 318: 26.10	-
NO	9B. INSPECTION OF PRESTRESSED CONCRETE: GROUTING OF BONDED PRESTRESSING TENDONS.	X	-	ACI 318: 26.10	-
YES	10. INSPECTION OF ERECTION OF PRECAST MEMBERS	-	X	ACI 318: Ch. 26.9	-
YES	11A. FOR PRECAST CONCRETE DIAPHRAGM CONNECTIONS OR REINFORCEMENT AT JOINTS CLASSIFIED AS MODERATE OR HIGH DEFORMABILITY ELEMENTS (MDE OR HDE) IN STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY C, D, E OR F, INSPECT SUCH CONNECTIONS AND REINFORCEMENT IN THE FIELD FOR: INSTALLATION OF THE EMBEDDED PARTS	X	-	ACI 318: Ch. 26.13.1.3 ACI 550.5	-
YES	11B. FOR PRECAST CONCRETE DIAPHRAGM CONNECTIONS OR REINFORCEMENT AT JOINTS CLASSIFIED AS MODERATE OR HIGH DEFORMABILITY ELEMENTS (MDE OR HDE) IN STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY C, D, E OR F, INSPECT SUCH CONNECTIONS AND REINFORCEMENT IN THE FIELD FOR: COMPLETION OF THE CONTINUITY OF REINFORCEMENT ACROSS JOINTS.	X	-	ACI 318: Ch. 26.13.1.3 ACI 550.5	-
YES	11C. FOR PRECAST CONCRETE DIAPHRAGM CONNECTIONS OR REINFORCEMENT AT JOINTS CLASSIFIED AS MODERATE OR HIGH DEFORMABILITY ELEMENTS (MDE OR HDE) IN STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY C, D, E OR F, INSPECT SUCH CONNECTIONS AND REINFORCEMENT IN THE FIELD FOR: COMPLETION OF CONNECTIONS IN THE FIELD.	X	-	ACI 318: Ch. 26.13.1.3 ACI 550.5	-
NO	12. INSPECT INSTALLATION TOLERANCES OF PRECAST CONCRETE DIAPHRAGM CONNECTIONS FOR COMPLIANCE WITH ACI 550.5	-	X	ACI 318: Ch. 26.13.1.3	-
YES	13. VERIFICATION OF IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS	-	X	ACI 318: 26.11.2	-
YES	14. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED	-	X	ACI 318: 26.11.1.2(b)	-

a. WHERE APPLICABLE, SEE ALSO SECTION 1705.12. SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE.

b. SPECIFIC REQUIREMENTS FOR SPECIAL INSPECTION SHALL BE INCLUDED IN THE RESEARCH REPORT FOR THE ANCHOR ISSUED BY AN APPROVED SOURCE IN ACCORDANCE WITH 17.8.2 IN ACI318, OR OTHER QUALIFICATION PROCEDURES. WHERE SPECIFIC REQUIREMENTS ARE NOT PROVIDED, SPECIAL INSPECTION REQUIREMENTS SHALL BE SPECIFIED BY THE REGISTERED DESIGN PROFESSIONAL AND SHALL BE APPROVED BY THE BUILDING OFFICIAL PRIOR TO COMMENCEMENT OF THE WORK.

No.	Date	Issue / Revision

CODY LAMBERT, State of Florida, Professional Engineer, License No. 100455

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FLORIDA-ALABAMA TPO

ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

GENERAL NOTES

DWG S-005 SHEET

AISC 360-16 (TABLES N5.4-1, N5.4-2, AND N5.4-3)
REQUIRED VERIFICATION AND INSPECTION TASKS FOR WELDING OF STRUCTURAL STEEL

SPECIAL INSPECTION REQUIRED	VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	REFERENCED STANDARD	IBC REFERENCE
	1. INSPECTION TASKS PRIOR TO WELDING				
YES	A. WELDER QUALIFICATION RECORDS AND CONTINUITY RECORDS	X	-	AISC 360-16: N5.4-1; AWS D1.1	1705.2.1
YES	B. WPS AVAILABLE	X	-	AISC 360-16: N5.4-1; AWS D1.1	1705.2.1
YES	C. MANUFACTURER CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE	X	-	AISC 360-16: N5.4-1; AWS D1.1	1705.2.1
YES	D. MATERIAL IDENTIFICATION (TYPE/GRADE)	-	X	AISC 360-16: N5.4-1; AWS D1.1	1705.2.1
YES	E. WELDER IDENTIFICATION SYSTEM	-	X	AISC 360-16: N5.4-1; AWS D1.1	1705.2.1
YES	F. FIT-UP GROOVE WELDS (INCLUDING JOINT GEOMETRY) I. JOINT PREPARATION II. DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL) III. CLEANLINESS (CONDITION OF STEEL SURFACES) IV. TACKING (TACK WELD QUALITY AND LOCATION)	X	-	AISC 360-16: N5.4-1; AWS D1.1	1705.2.1
YES	G. CONFIGURATION AND FINISH OF ACCESS HOLES	-	X	AISC 360-16: N5.4-1; AWS D1.1	1705.2.1
YES	H. FIT-UP FILLET WELDS I. DIMENSIONS (ALIGNMENT, GAPS AT ROOT) II. CLEANLINESS (CONDITION OF STEEL SURFACES) III. TACKING (TACK WELD QUALITY AND LOCATION)	-	X	AISC 360-16: N5.4-1; AWS D1.1	1705.2.1
YES	I. CHECK WELDING EQUIPMENT	-	X	AISC 360-16: N5.4-1; AWS D1.1	1705.2.1
	2. INSPECTION TASKS DURING WELDING				
YES	A. CONTROL AND HANDLING OF WELDING CONSUMABLES I. PACKAGING II. EXPOSURE CONTROL	-	X	AISC 360-16: N5.4-2; AWS D1.1	1705.2.1
YES	B. NO WELDING OVER CRACKED TACK WELDS	-	X	AISC 360-16: N5.4-2; AWS D1.1	1705.2.1
YES	C. ENVIRONMENTAL CONDITIONS I. WIND SPEED WITHIN LIMITS II. PRECIPITATION AND TEMPERATURE	-	X	AISC 360-16: N5.4-2; AWS D1.1	1705.2.1
YES	D. WPS FOLLOWED I. SETTINGS ON WELDING EQUIPMENT II. TRAVEL SPEED III. SELECTED WELDING MATERIALS IV. SHIELDING GAS TYPE/FLOW RATE V. PREHEAT APPLIED VI. INTERPASS TEMPERATURE MAINTAINED (MIN./MAX.) VII. PROPER POSITION (F, V, H, OH)	-	X	AISC 360-16: N5.4-2; AWS D1.1	1705.2.1
YES	E. WELDING TECHNIQUES I. INTERPASS AND FINAL CLEARING II. EACH PASS WITHIN PROFILE LIMITATIONS III. EACH PASS MEETS QUALITY REQUIREMENTS	-	X	AISC 360-16: N5.4-2; AWS D1.1	1705.2.1
YES	F. PLACEMENT AND INSTALLATION OF STEEL HEADED STUD ANCHORS	X	-	AISC 360-16: N5.4-2; AWS D1.1	1705.2.1
	3. INSPECTION TASKS AFTER WELDING				
YES	A. WELDS CLEANED	-	X	AISC 360-16: N5.4-3; AWS D1.1	1705.2.1
YES	B. SIZE, LENGTH AND LOCATION OF WELDS	X	-	AISC 360-16: N5.4-3; AWS D1.1	1705.2.1
YES	C. WELDS MEET VISUAL ACCEPTABLE CRITERIA I. CRACK PROHIBITION II. WELD/BASE-METAL FUSION III. CRATER CROSS SECTION IV. WELD PROFILES V. WELD SIZE VI. UNDERCUT VII. POROSITY	X	-	AISC 360-16: N5.4-3; AWS D1.1	1705.2.1
YES	D. ARC STRIKES	X	-	AISC 360-16: N5.4-3; AWS D1.1	1705.2.1
YES	E. K-AREA2	X	-	AISC 360-16: N5.4-3; AWS D1.1	1705.2.1
YES	F. WELD ACCESS HOLES IN ROLLED HEAVY SHAPES AND BUILT-UP HEAVY SHAPES				
YES	G. BACKING REMOVED AND WELD TABS REMOVED (IF REQUIRED)	X	-	AISC 360-16: N5.4-3; AWS D1.1	1705.2.1
YES	H. REPAIR ACTIVITIES	X	-	AISC 360-16: N5.4-3; AWS D1.1	1705.2.1
YES	I. DOCUMENT ACCEPTANCE OR REJECTION OF WELDED JOINT OR MEMBER	X	-	AISC 360-16: N5.4-3; AWS D1.1	1705.2.1
YES	J. NO PROHIBITED WELDS HAVE BEEN ADDED WITHOUT THE APPROVAL OF THE EOR	-	X	AISC 360-16: N5.4-3; AWS D1.1	1705.2.1

1. THE FABRICATOR OR ERECTOR, AS APPLICABLE, SHALL MAINTAIN A SYSTEM BY WHICH A WELDER WHO HAS WELDED A JOINT OR MEMBER CAN BE INDENTIFIED. STAMPS, IF USED SHALL BE THE LOW-STRESS TYPE.
2. WHEN WELDING OF DOUBLER PLATES, CONTINUITY PLATES OR STIFFENERS HAS BEEN PERFORMED IN THE K-AREA, VISUALLY INSPECT THE WEB K-AREA FOR CRACKS WITHIN 3 INCHES OF THE WELD.
3. AFTER ROLLED HEAVY SHAPES (SEE SECTION A3.1C) AND BUILT-UP HEAVY SHAPES (SEE SECTION A3.1D) ARE WELDED, VISUALLY INSPECT THE WELD ACCESS HOLE FOR CRACKS.

TABLE 1705.2.3
REQUIRED VERIFICATION AND INSPECTION OF OPEN-WEB STEEL JOINTS AND JOIST GIRDERS

SPECIAL INSPECTION REQUIRED	VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	REFERENCED STANDARDa
	1. INSTALLATION OF OPEN-WEB STEEL JOISTS AND JOIST GIRDERS.			
YES	A. END CONNECTIONS-WELDING OR BOLTED.	-	X	SJI SPECIFICATION LISTED IN SECTION 2207.1.
YES	B. BRIDGING- HORIZONTAL OR DIAGONAL.	-	-	
	I. STANDARD BRIDGING	-	X	SJI SPECIFICATION LISTED IN SECTION 2207.1.
	II. BRIDGING THAT DIFFERS FROM THE SJI SPECIFICATIONS LISTED IN SECTION 2207.1.	-	X	-

a. WHERE APPLICABLE, SEE ALSO SECTION 1705.12, SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE

TABLE 1705.2.2
REQUIRED VERIFICATION AND INSPECTION OF STEEL CONSTRUCTION OTHER THAN STRUCTURAL STEEL

SPECIAL INSPECTION REQUIRED	VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	REFERENCED STANDARD
	1. MATERIAL VERIFICATION OF COLD-FORMED STEEL DECK			
YES	A. IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS	-	X	APPLICABLE ASTM MATERIAL STANDARDS
YES	B. MANUFACTURER'S CERTIFIED TEST REPORTS	-	X	
	2. INSPECTION OF WELDING			
YES	A. COLD-FORMED STEEL DECK I. FLOOR AND ROOF DECK WELDS	-	X	AWS D1.3
	B. REINFORCING STEEL			
YES	I. VERIFICATION OF WELDABILITY OF REINFORCING STEEL OTHER THAN ASTM A 706	-	X	AWS D1.4 ACI 318: 3.5.2
YES	II. REINFORCING STEEL RESISTING FLEXURAL AND AXIAL FORCES IN INTERMEDIATE AND SPECIAL MOMENT FRAMES, AND BOUNDARY ELEMENTS OF SPECIAL STRUCTURAL WALLS OF CONCRETE AND SHEAR REINFORCEMENT	X	-	AWS D1.4 ACI 318: 3.5.2
YES	III. SHEAR REINFORCEMENT	X	-	AWS D1.4 ACI 318: 3.5.2
YES	IV. OTHER REINFORCING STEEL	-	X	AWS D1.4 ACI 318: 3.5.2

ACI 530-11/TMS 402-11 TABLE 1.19.2
LEVEL B QUALITY ASSURANCE

SPECIAL INSPECTION REQUIRED	MINIMUM TESTS				
YES	VERIFICATION OF SLUMP FLOW AND VISUAL STABILITY INDEX (VSI) AS DELIVERED TO THE PROJECT SITE IN ACCORDANCE WITH SPECIFICATION ARTICLE 1.5 B.1.B.3 FOR SELF-CONSOLIDATING GROUT				
YES	VERIFICATION OF F'_M AND F'_{AAC} IN ACCORDANCE WITH SPECIFICATION ARTICLE 1.4 B PRIOR TO CONSTRUCTION, EXCEPT WHERE SPECIFICALLY EXEMPTED BY THIS CODE				
	INSPECTION TASK	FREQUENCY		REFERENCE FOR CRITERIA	
		CONTINUOUS	PERIODIC	TMS 402 / ACI 530 / ASCE 5	TMS 602 / ACI 530.1 / ASCE 6
YES	1. VERIFY COMPLIANCE WITH THE APPROVED SUBMITTALS	-	X	-	Art. 1.5
	2. AS MASONRY CONSTRUCTION BEGINS, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:				
YES	A. PROPORTIONS OF SITE-PREPARED MORTAR	-	X	-	Art. 2.1, 2.6 A
YES	B. MANUFACTURER'S CERTIFIED TEST REPORTS	-	X	-	Art. 3.3 B
NO	C. GRADE AND SIZE OF PRESTRESSING TENDONS AND ANCHORAGES	-	X	-	Art. 2.4 B, 2.4 H
YES	D. LOCATION OF REINFORCEMENT, CONNECTORS, AND PRESTRESSING TENDONS AND ANCHORAGES	-	X	-	Art. 3.4, 3.6 A
NO	E. PRESTRESSING TECHNIQUE	-	X	-	Art. 3.6 B
NO	F. PROPERTIES OF THIN-BED MORTAR FOR AAC MASONRY	X[a]	X[b]	-	Art. 2.1 C
	3. PRIOR TO GROUTING, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:				
YES	A. GROUT SPACE	-	X	-	Art. 3.2 D, 3.2 F
YES	B. GRADE, TYPE, AND SIZE OF REINFORCEMENT AND ANCHOR BOLTS, AND PRESTRESSING TENDONS AND ANCHORAGES	-	X	Sec. 1.16	Art. 2.4, 3.4
YES	C. PLACEMENT OF REINFORCEMENT, CONNECTORS, AND PRESTRESSING TENDONS AND ANCHORAGES	-	X	Sec. 1.16	Art. 3.2 E, 3.4, 3.6 A
YES	D. PROPORTIONS OF SITE-PREPARED GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS	-	X	-	Art. 2.6 B, 2.4 G.1.b
YES	E. CONSTRUCTION OF MORTAR JOINTS	-	X	-	Art. 3.3 B
	4. VERIFY DURING CONSTRUCTION:				
YES	A. SIZE AND LOCATION OF STRUCTURAL ELEMENTS	-	X	-	Art. 3.3 F
YES	B. TYPE, SIZE, AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES, OR OTHER CONSTRUCTION	-	X	Sec 1.16.4.3, 1.17.1	-
NO	C. WELDING OF REINFORCEMENT	X	-	Sec. 2.1.7.7.2, 3.3.3.4 (c), 8.3.3.4 (b)	-
YES	D. PREPARATION, CONSTRUCTION, AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40 DEGREES F OR HOT WEATHER (TEMPERATURE ABOVE 90 DEGREES F)	-	X	-	Art. 1.8 C, 1.8 D
NO	E. APPLICATION OF MEASUREMENT OF PRESTRESSING FORCE	X	-	-	Art. 3.6 B
NO	F. PLACEMENT OF GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS IS IN COMPLIANCE	X	-	-	Art. 3.5, 3.6 C
NO	G. PLACEMENT OF AAC MASONRY UNITS AND CONSTRUCTION OF THIN-BED MORTAR JOINTS	X[a]	X[b]	-	Art. 3.3 B.8
YES	5. OBSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS	-	X	-	Art. 1.4 B.2.a.3, 1.4 B.2.b.3, 1.4 B.2.c.3, 1.4 B.3, 1.4 B.4

a. REQUIRED FOR THE FIRST 5000 SQUARE FEET OF AAC MASONRY
b. REQUIRED AFTER THE FIRST 5000 SQUARE FEET OF AAC MASONRY

ABBREVIATIONS

ADDL	=	ADDITIONAL
AESS	=	ARCHITECTURALLY EXPOSED STRUCTURAL STEEL
ALT	=	ALTERNATE
ARCH	=	ARCHITECTURAL
BO ___	=	BOTTOM OF ___
BOT. B	=	BOTTOM
BRG	=	BEARING
BTW	=	BETWEEN
CGS	=	CENTER OF GRAVITY OF STEEL
CIP	=	CAST-IN-PLACE
CJ	=	CONTROL JOINT
CJP	=	COMPLETE JOINT PENETRATION
CL	=	CENTER LINE
CLR	=	CLEAR
CMU	=	CONCRETE MASONRY UNIT
COL	=	COLUMN
CONC	=	CONCRETE
CONN	=	CONNECTION
CONT	=	CONTINUOUS
DBA	=	DEFORMED BAR ANCHOR
DET	=	DETAIL
DIA	=	DIAMETER
DIAG	=	DIAGONAL
DWL	=	DOWEL
EA	=	EACH
EC	=	EPOXY COATED
EE	=	EACH END
EF	=	EACH FACE
EJ	=	EXPANSION JOINT
EL	=	ELEVATION
ELEC	=	ELECTRICAL
EOR	=	ENGINEER OF RECORD
EOS	=	EDGE OF SLAB
EQ	=	EQUAL
EW	=	EACH WAY
E/W	=	EAST/WEST
EXIST	=	EXISTING
FD	=	FLOOR DRAIN
FDN	=	FOUNDATION
FS	=	FAR SIDE
FT	=	FOOT
FTG	=	FOOTING
FV	=	FIELD VERIFY
GA	=	GAUGE
GALV	=	GALVANIZED
GC	=	GENERAL CONTRACTOR
GR	=	GRADE
HDPE	=	HIGH DENSITY POLYETHYLENE
HK	=	HOOK
HORIZ	=	HORIZONTAL
HSA	=	HEADED STUD ANCHOR
HSS	=	HOLLOW STRUCTURAL SECTION
IF	=	INSIDE FACE
IP	=	INFLECTION POINT
K	=	KIP
LLH	=	LONG LEG HORIZONTAL
LLV	=	LONG LEG VERTICAL
LSH	=	LONG SIDE HORIZONTAL
LSL	=	LONG SLOT
LSV	=	LONG SIDE VERTICAL
LT WALL	=	LIGHT WALL
LW	=	LIGHTWEIGHT
MAX	=	MAXIMUM
MECH	=	MECHANICAL
MFR	=	MANUFACTURER
MIN	=	MINIMUM
NOM	=	NOMINAL
NS	=	NEAR SIDE
NSNS	=	NON-SHRINK, NON-STAIN
N/S	=	NORTH/SOUTH
NTS	=	NOT TO SCALE
OC	=	ON CENTER
OF	=	OUTSIDE FACE
OH	=	OPPOSITE HAND
OVS	=	OVERSIZED
PCC	=	PRECAST CONCRETE
PJP	=	PARTIAL JOINT PENETRATION
PL	=	PLATE
PT	=	POST-TENSIONED
QTY	=	QUANTITY
R	=	REMAINDER
RD	=	ROOF DRAIN
REINF	=	REINFORCING
REQ'D	=	REQUIRED
RFI	=	REQUEST FOR INFORMATION
SIM	=	SIMILAR
SOG	=	SLAB-ON-GRADE
SPEC	=	SPECIFICATION
SS	=	STAINLESS STEEL
SSL	=	SHORT SLOT
STD	=	STANDARD
STIR	=	STIRRUPS
STL	=	STEEL
STR	=	STRUCTURE
T	=	TOP
T&B	=	TOP AND BOTTOM
THK	=	THICK
TO ___	=	TOP OF ___
TOB	=	TOP OF BEAM
TOC	=	TOP OF CONCRETE
TOF	=	TOP OF FOUNDATION/ FOOTING
TOS	=	TOP OF STEEL
TOW	=	TOP OF WALL
TEMP	=	TEMPORARY
TYP	=	TYPICAL
UNO	=	UNLESS NOTED OTHERWISE
VERT	=	VERTICAL
W/	=	WITH
W/O	=	WITHOUT
WF	=	WIDE FLANGE
WWR	=	WELDED WIRE REINFORCEMENT

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No.	Date	Issue / Revision

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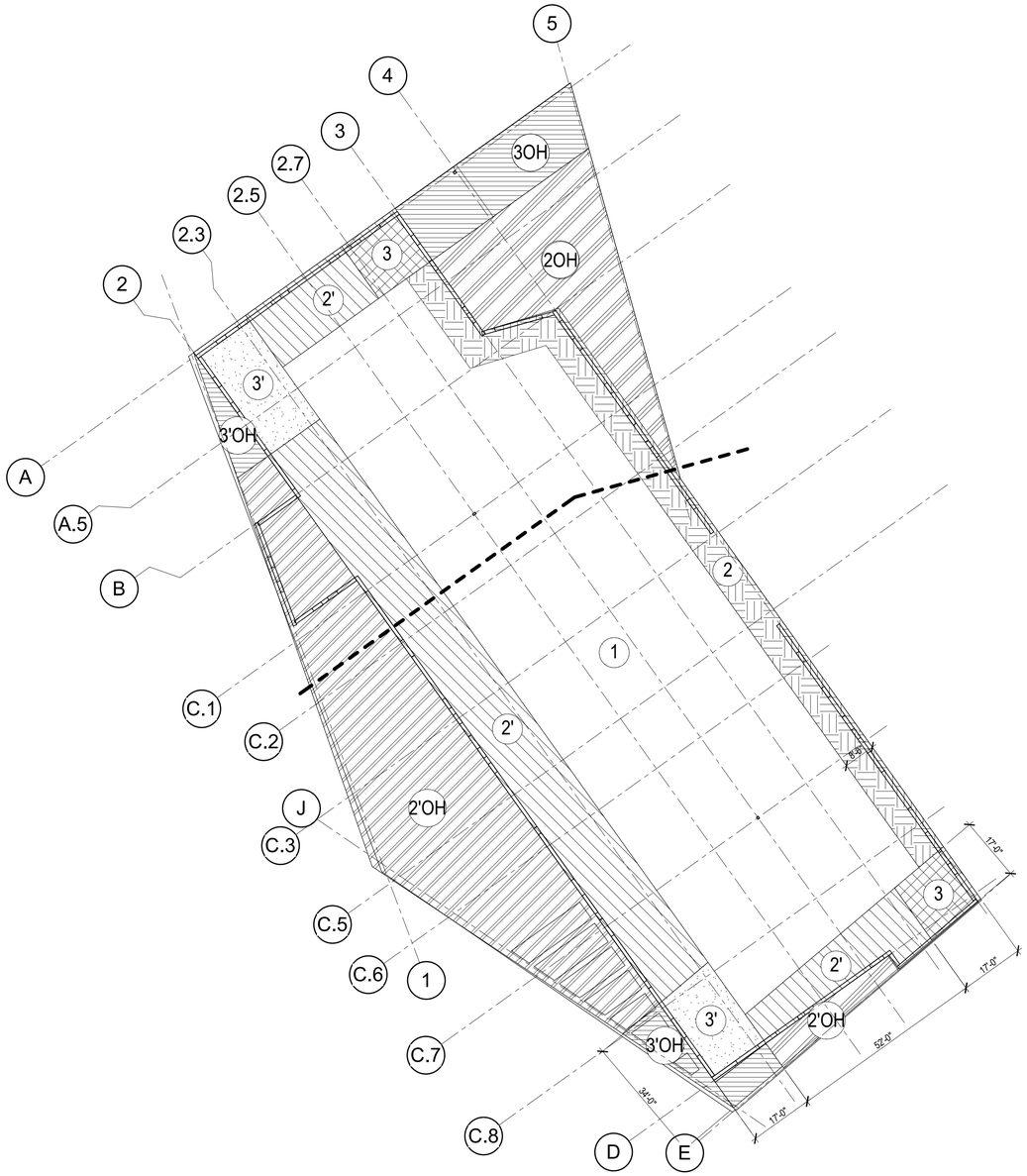
FLORIDA-ALABAMA TPO

ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

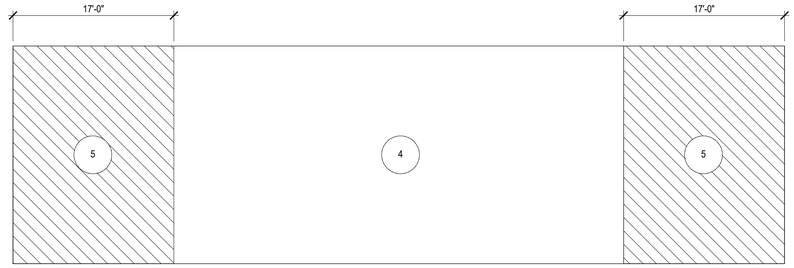
GENERAL NOTES

DWG
S-006
SHEET



1 C&C ROOF ZONE DIAGRAM

NOT TO SCALE



3 C&C WALL ZONE DIAGRAM

NOT TO SCALE

COMPONENTS AND CLADDING ROOF PRESSURES			
COMPONENT EFFECTIVE AREA	ZONE	WIND PRESSURE (ULTIMATE) POSITIVE (PSF)	NEGATIVE (PSF)
<10 SF	1	32.4	-86.5
20 SF	1	30.4	-86.5
50 SF	1	27.7	-86.5
100 SF	1	25.7	-86.5
<10 SF	2	32.4	-100.0
20 SF	2	30.4	-97.9
50 SF	2	27.7	-95.2
100 SF	2	25.7	-93.2
<10 SF	2'	32.4	-120.2
20 SF	2'	30.4	-118.2
50 SF	2'	27.7	-115.5
100 SF	2'	25.7	-113.5
<10 SF	3	32.4	-133.7
20 SF	3	30.4	-121.5
50 SF	3	27.7	-105.4
100 SF	3	25.7	-93.2
<10 SF	3'	32.4	-187.8
20 SF	3'	30.4	-167.4
50 SF	3'	27.7	-140.6
100 SF	3'	25.7	-120.2
<10 SF	2OH	32.4	-154.7
20 SF	2OH	30.4	-149.4
50 SF	2OH	27.7	-142.4
100 SF	2OH	25.7	-137.2
<10 SF	2'OH	32.4	-174.9
20 SF	2'OH	30.4	-169.7
50 SF	2'OH	27.7	-162.7
100 SF	2'OH	25.7	-157.4
<10 SF	3OH	32.4	-206.7
20 SF	3OH	30.4	-188.0
50 SF	3OH	27.7	-163.3
100 SF	3OH	25.7	-144.7
<10 SF	3'OH	32.4	-260.7
20 SF	3'OH	30.4	-233.9
50 SF	3'OH	27.7	-198.5
100 SF	3'OH	25.7	-171.7

COMPONENTS AND CLADDING WALL PRESSURES			
COMPONENT EFFECTIVE AREA	ZONE	WIND PRESSURE (ULTIMATE) POSITIVE (PSF)	NEGATIVE (PSF)
<10 SF	4	72.9	-79.0
20 SF	4	69.7	-75.8
50 SF	4	65.4	-71.5
100 SF	4	62.2	-68.3
<10 SF	5	72.9	-97.3
20 SF	5	69.7	-90.8
50 SF	5	65.4	-82.3
100 SF	5	62.2	-75.8

2 C&C ZONE DESIGN VALUES

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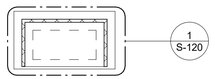
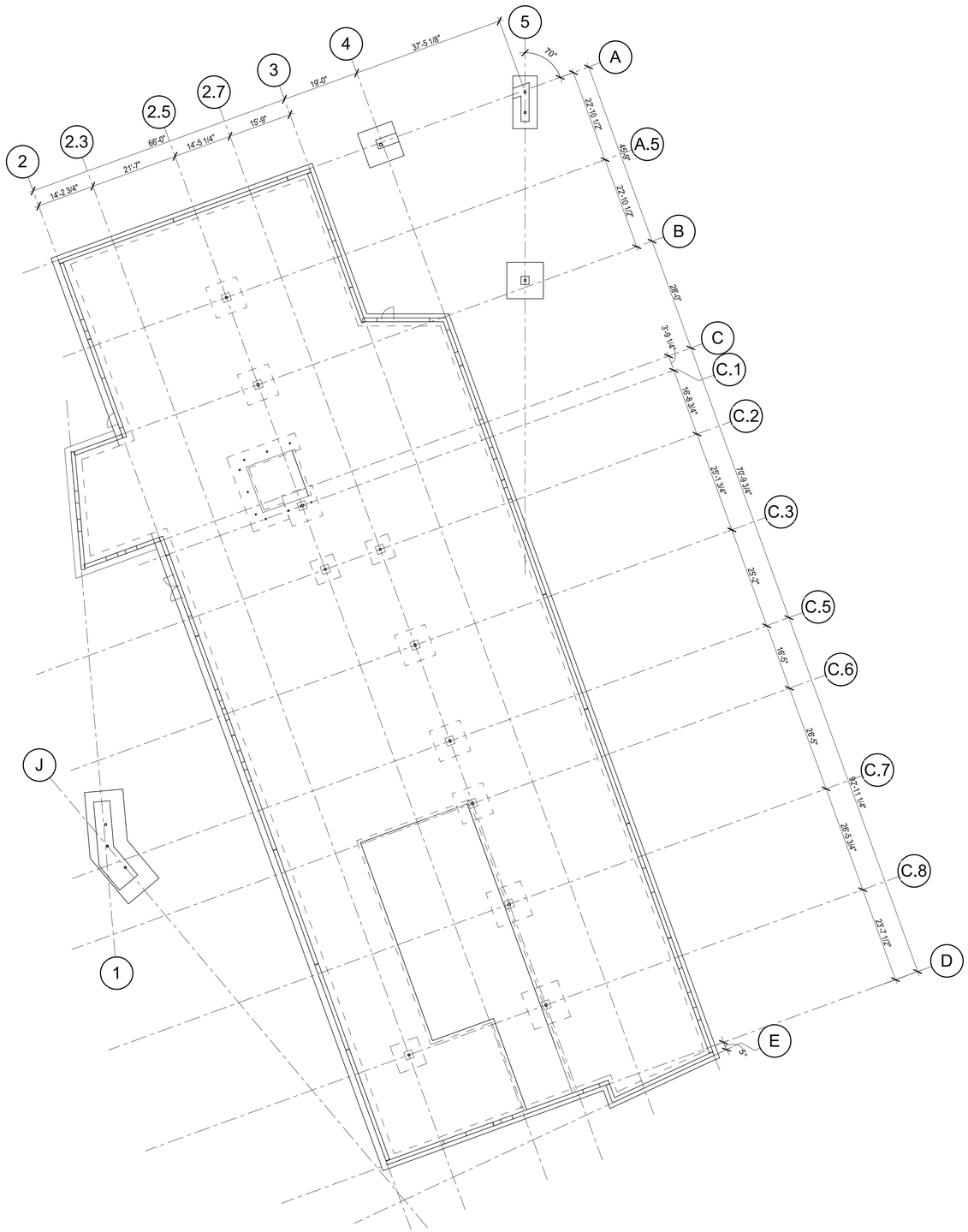
FLORIDA-ALABAMA TPO

ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

WIND PRESSURES AND DIAGRAMS

DWG
S-007
SHEET





1 FOUNDATION PLAN - OVERALL

SCALE: 1/16" = 1'-0"

No.	Date	Issue / Revision

CODY LAMBERT, State of Florida, Professional Engineer, License No. 100455

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WGI, INC.
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TAMPA, FL 33607

FLORIDA-ALABAMA TPO

ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

FOUNDATION PLAN - OVERALL

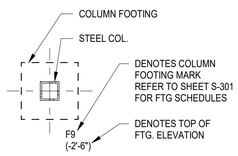
DWG S-100

SHEET

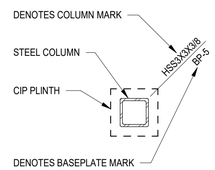
FOUNDATION NOTES:

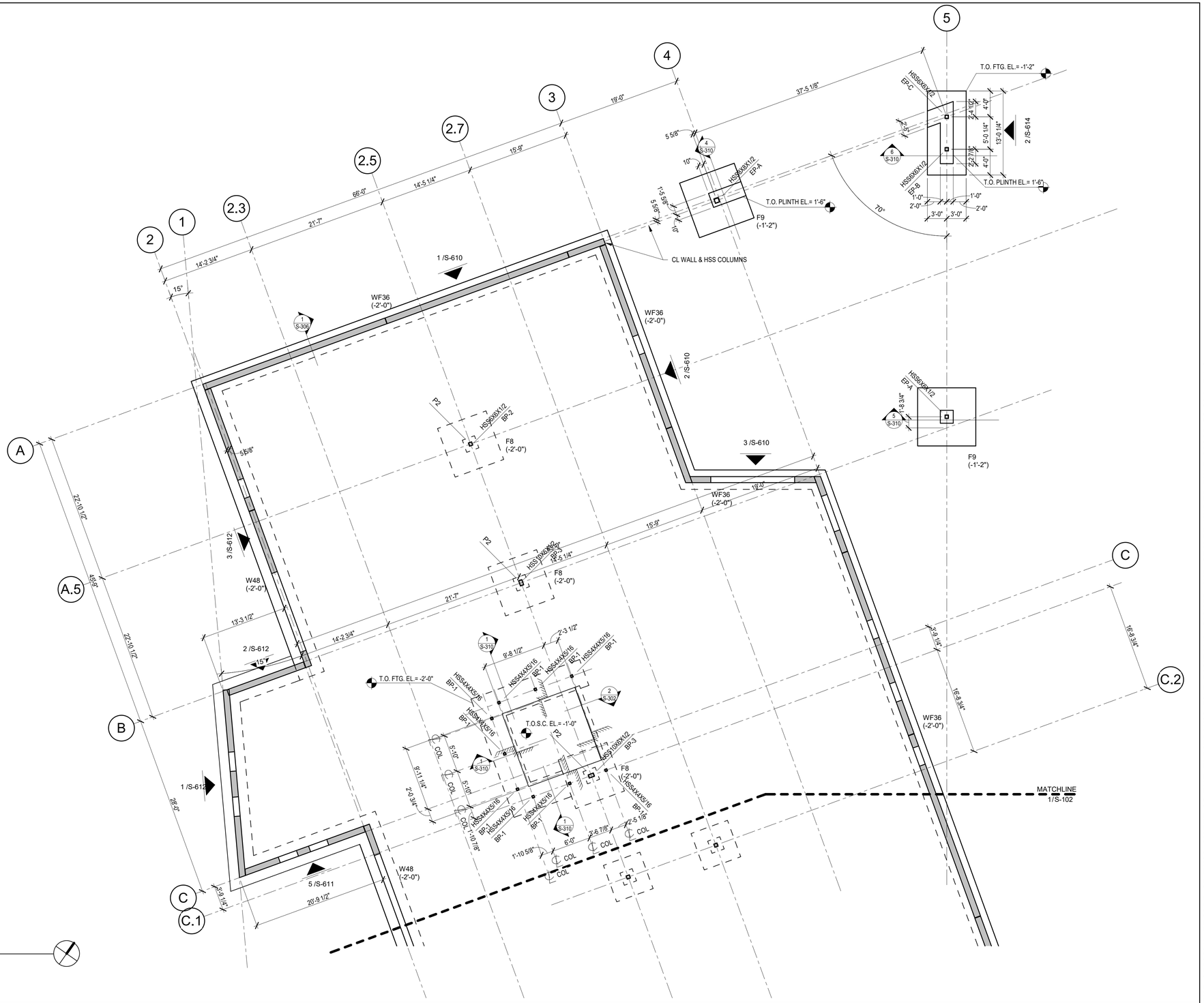
1. SLAB SHALL BE 5" THK. REINF. W/ #4 @ 16" O.C. E.W. SUBGRADE SHALL BE PREPARED IN ACCORDANCE WITH THE GEOTECHNICAL REPORT RECOMMENDATIONS.
2. FINISH FLOOR ELEVATION = 0'-0" (NAVD 88 1'16'-0") , UNLESS NOTED OTHERWISE.REF. TO CIVIL FOR ADDITIONAL INFORMATION.
3. REFER TO THE ARCHITECTURAL DRAWINGS FOR EXACT LOCATIONS OF FLOOR RECESSES, DROPS AND SLOPES NOT DIMENSIONED ON PLAN. REFER TO MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR LOCATION AND DIMENSIONS OF FLOOR PENETRATIONS NOT DIMENSIONED ON PLAN. CONTRACTOR TO COORDINATE.
4. CENTERLINES OF FOUNDATIONS NOT SPECIFICALLY LOCATED ON PLAN BY NOTE OR DIMENSION SHALL BE LOCATED AS FOLLOWS:
   A. SUPPORTING FREESTANDING COLUMNS: CENTERLINES OF COLUMN.
   B. SUPPORTING GRADEBEAMS AND WALLS: CENTERLINE OF GRADEBEAM OR WALL IN ONE DIRECTION, GRID OR AS NOTED IN OTHER DIRECTION. AT CORNER CONDITIONS: CENTERLINES OF GRADEBEAMS OR WALLS.
   C. COLUMNS EMBEDED IN GRADEBEAMS OR WALLS (PILASTERS): CENTERLINES OF THE COLUMN.
5. TYPICAL CONCRETE SLAB THICKNESS IS 5" (OVERALL), UNLESS NOTED OTHERWISE.
6. REF. SHEETS S-301 - S-304 FOR ADD'L SLAB-ON-GRADE INFORMATION

ISOLATED FOOTING NOTES:



STEEL COLUMN NOTES:





1 FOUNDATION PLAN - NORTH
SCALE: 1/8" = 1'-0"

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TAMPA, FL 33607

FLORIDA-ALABAMA TPO

ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

FOUNDATION PLAN - NORTH

DWG S-101 SHEET

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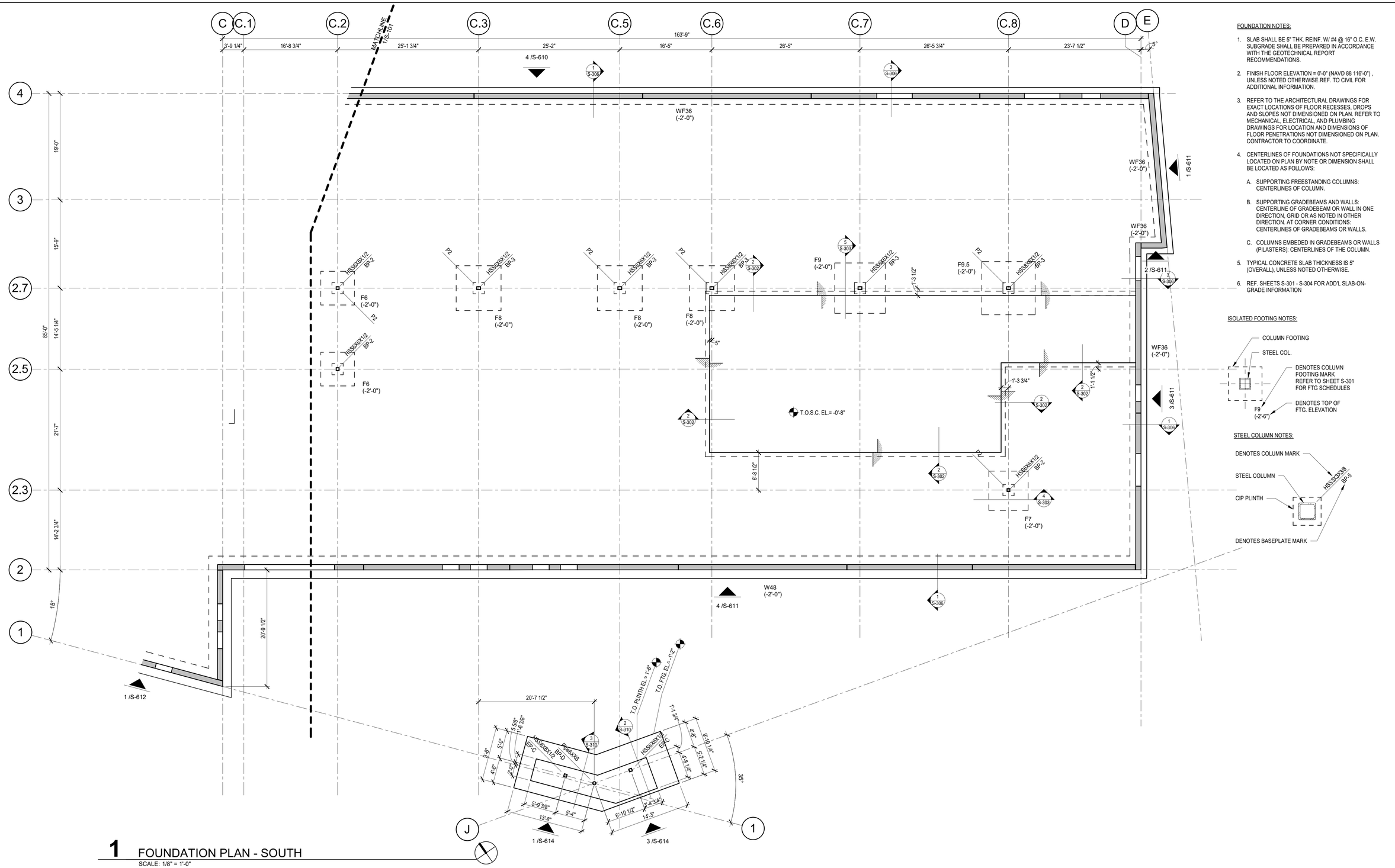

1 FOUNDATION PLAN - SOUTH

SCALE: 1/8" = 1'-0"

No.	Date	Issue / Revision

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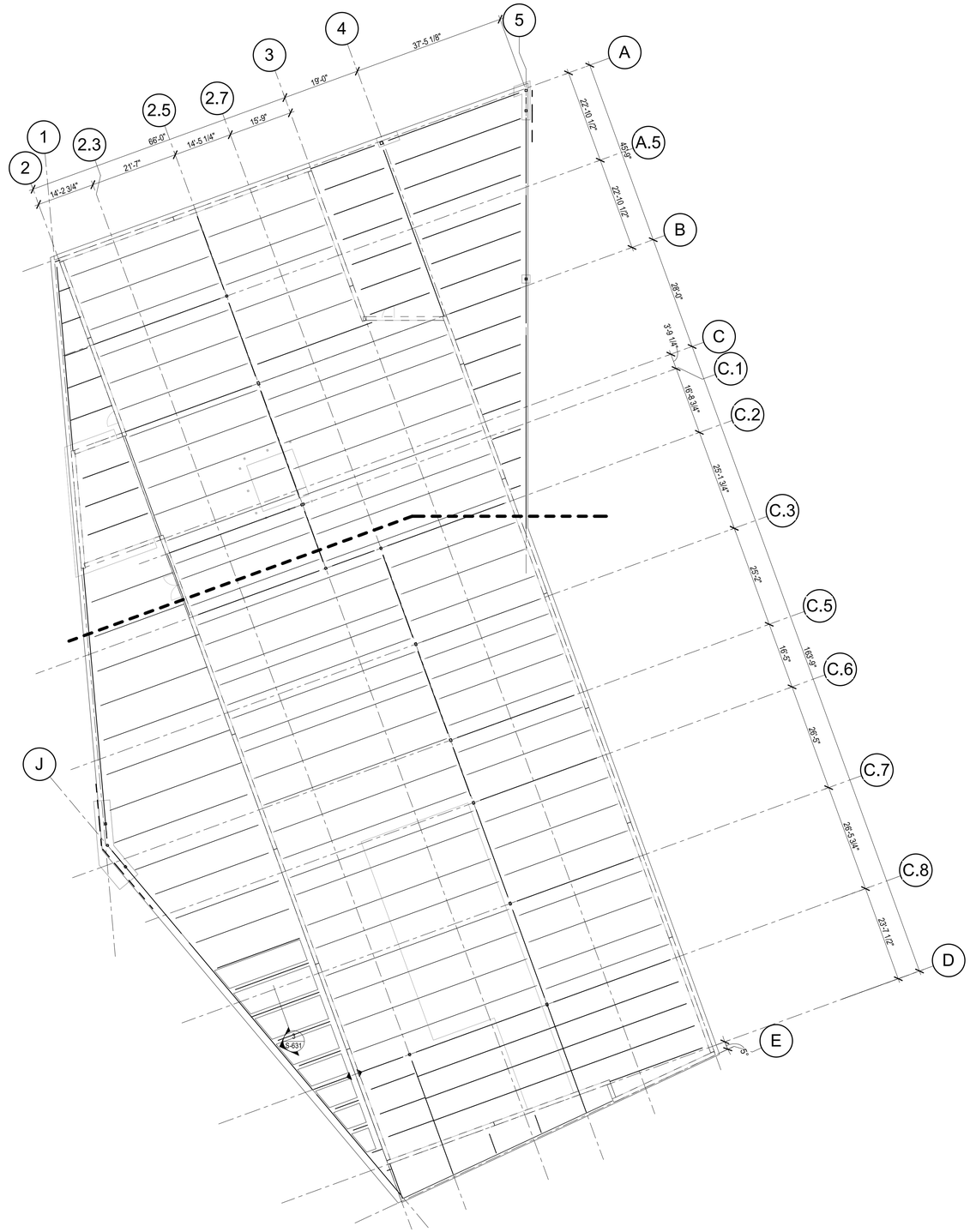
FLORIDA-ALABAMA TPO

ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

FOUNDATION PLAN - SOUTH

DWG S-102 SHEET



1 ROOF FRAMING PLAN - OVERALL

SCALE: 1/16" = 1'-0"

No.	Date	Issue / Revision

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FLORIDA-ALABAMA TPO

ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

ROOF FRAMING PLAN - OVERALL

DWG S-110

SHEET

STEEL FRAMING NOTES:

1. REF. S-001 - S-007 FOR STRUCTURAL NOTES, DESIGN DATA, SCHEDULES & LEGENDS.
2. REF. THE S-550 SERIES FOR TYPICAL FRAMING DETAILS.
3. ALL CONTRACTORS ARE REQUIRED TO COORDINATE THEIR WORK WITH ALL DISCIPLINES TO AVOID CONFLICTS. THE MECHANICAL, ELECTRICAL, AND PLUMBING ASPECTS ARE NOT IN THE SCOPE OF THESE DRAWINGS. THEREFORE, ALL REQUIRED MATERIALS AND WORK MAY NOT BE INDICATED.
4. ALL ELEVATIONS ARE REFERENCED FROM THE FIRST FLOOR FINISHED FLOOR ELEVATION +0' - 0" (NAVD 88 116'-0"). REF CIVIL DWGS.
5. ALL WALLS SHALL BE LAID OUT FROM THE ARCHITECTURAL DRAWINGS.
6. REF. ARCH. DRAWINGS. FOR ALL DIMENSIONS NOT SHOWN. CONTRACTOR SHALL VERIFY ALL DIMENSIONS PRIOR TO CONSTRUCTION AND IMMEDIATELY NOTIFY ARCHITECT/ENGINEER OF ANY DISCREPANCIES.
7. COORDINATE EXACT SIZE & LOCATION OF ANY MECHANICAL OPENINGS IN FLOOR SLAB, ROOF DECK, OR WALLS WITH THE M.E.P. CONTRACTOR. LOCATION & SIZE OF ALL DUCT OPENINGS, GRILLS, ETC. SHALL BE VERIFIED PRIOR TO CONSTRUCTION.
8. ALL ELEVATIONS SHOWN ON PLAN INDICATE TOP OF = BOTTOM OF DECK UNLESS NOTED OTHERWISE.
9. PROVIDE FRAMES AT ALL ROOF DRAINS, ROOF HATCHES & OTHER ROOF OPENINGS PER TYPICAL DETAILS ON S-551 & S-552 COORDINATE EXACT NUMBER, LOCATIONS & DIMENSIONS WITH THE APPROPRIATE CONTRACTORS & THE ARCH. & M.E.P. DRAWINGS.
10. ALL HORIZONTAL AND DIAGONAL BRIDGING FOR STEEL JOISTS SHALL BE DESIGNED, LOCATED & PROVIDED BY THE JOIST SUPPLIER PER SJI SPECIFICATIONS.
11. WIDE-FLANGE BEAM & GIRDER NOTATION:BEAM REACTIONS SHOWN IN KIPS (LRFD / FACTORED LOADS) TO BE USED FOR DESIGN OF SHEAR CONNECTION BY STEEL FABRICATOR. SIMPLE SPAN CONNECTIONS FOR BEAMS & GIRDERS LESS THAN 10 FEET IN LENGTH AND WITH NO REACTION SHOWN SHALL BE DESIGNED FOR 10 KIPS MINIMUM. SEE STEEL CONNECTION NOTES ON SHEET S-551 FOR ADDITIONAL INFORMATION.
12. ALL FRAMING DIRECTLY SUPPORTING COMPOSITE METAL ROOF DECK SHALL HAVE HEADED STUD ANCHORS WELDED ALONG SPAN SPACED AT 18" OC MAX UNO.

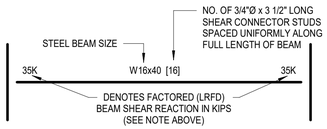
F.F. DENOTES FIN. FLOOR
T.O.S. EL. DENOTES TOP OF STEEL, SLAB, PIER, ETC.
BOT. EL DENOTES BOTTOM OF LINTEL, ETC.
DE DENOTES DECK EDGE DIMENSION

(S) DENOTES 20 GA. 2VLI DECK W/ 2" NW CONCRETE (4" TOTAL THICKNESS) DECK REINF. w/ 6X6 - W1.4XW1.4 WELDED WIRE MESH

DENOTES BEAM-TO-COLUMN MOMENT CONNECTION. REF. SPECIFIC SECTIONS & DETAILS

DENOTES BRACED FRAME OR KICKER LOCATION

DENOTES APPROX. LOCATION OF OPENING IN DECK/SLAB. REF. DETAILS ON S-551 & S-552 FOR TYPICAL OPENING FRAMES.



TYPICAL BEAM DIAGRAM

NOTE:
1. FOR MOMENT CONNECTIONS BETWEEN DIFFERENT-SIZED BEAMS, DESIGN CONNECTIONS FOR MAXIMUM MOMENT OF SMALLER BEAM
2. STUCTURAL STEEL FRAMING IS NOT DESIGNED TO EXHIBIT COMPOSITE BEHAVIOR. HEADED STUD ANCHORS ARE SPECIFIED TO TRANSFER UPLIFT FORCES TO PRIMARY FRAME.
3. REFER TO S-552 FOR ADDITIONAL INFORMATION RELATED TO HEADED STUD (SHEAR STUD) CONNECTORS

1 ROOF FRAMING PLAN - NORTH
SCALE: 1/8" = 1'-0"

No.	Date	Issue / Revision

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FLORIDA-ALABAMA TPO

ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

ROOF FRAMING PLAN - NORTH

DWG S-111 SHEET

STEEL FRAMING NOTES:

1. REF. S0-01 & S0-02 FOR STRUCTURAL NOTES, DESIGN DATA, SCHEDULES & LEGENDS.
2. REF. THE SD SERIES FOR TYPICAL FRAMING DETAILS.
3. ALL CONTRACTORS ARE REQUIRED TO COORDINATE THEIR WORK WITH ALL DISCIPLINES TO AVOID CONFLICTS. THE MECHANICAL, ELECTRICAL, AND PLUMBING ASPECTS ARE NOT IN THE SCOPE OF THESE DRAWINGS. THEREFORE, ALL REQUIRED MATERIALS AND WORK MAY NOT BE INDICATED.
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8. ALL ELEVATIONS SHOWN ON PLAN INDICATE TOP OF STEEL BEAM UNLESS NOTED OTHERWISE.
9. PROVIDE FRAMES AT ALL ROOF DRAINS, ROOF HATCHES & OTHER ROOF OPENINGS PER TYPICAL DETAILS ON S-551 & S-552 COORDINATE EXACT NUMBER, LOCATIONS & DIMENSIONS WITH THE APPROPRIATE CONTRACTORS & THE ARCH. & M.E.P. DRAWINGS.
10. NOTE: ALL HORIZONTAL AND DIAGONAL BRIDGING FOR STEEL JOISTS SHALL BE DESIGNED, LOCATED & PROVIDED BY THE JOIST SUPPLIER PER SJI SPECIFICATIONS.
11. WIDE-FLANGE BEAM & GIRDER NOTATION:BEAM REACTIONS SHOWN IN KIPS (LRFD / FACTORED LOADS) TO BE USED FOR DESIGN OF SHEAR CONNECTION BY STEEL FABRICATOR. SIMPLE SPAN CONNECTIONS FOR BEAMS & GIRDERS LESS THAN 10 FEET IN LENGTH AND WITH NO REACTION SHOWN SHALL BE DESIGNED FOR 10 KIPS MINIMUM. SEE STEEL CONNECTION NOTES ON SHEET S-551 FOR ADDITIONAL INFORMATION.
12. ALL FRAMING DIRECTLY SUPPORTING COMPOSITE METAL ROOF DECK SHALL HAVE HEADED STUD ANCHORS WELDED ALONG SPAN SPACED AT 18" OC MAX UNO.

F.F.	DENOTES FIN. FLOOR
T.O.S. EL.	DENOTES TOP OF STEEL, SLAB, PIER, ETC.
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(S)	DENOTES 20 GA. 2VLI DECK W/ 2" NW CONCRETE (4" TOTAL THICKNESS) DECK REINF. w/ 6X6 - W1.4XW1.4 WELDED WIRE MESH
▶━━	DENOTES BEAM-TO-COLUMN MOMENT CONNECTION. REF. SPECIFIC SECTIONS & DETAILS
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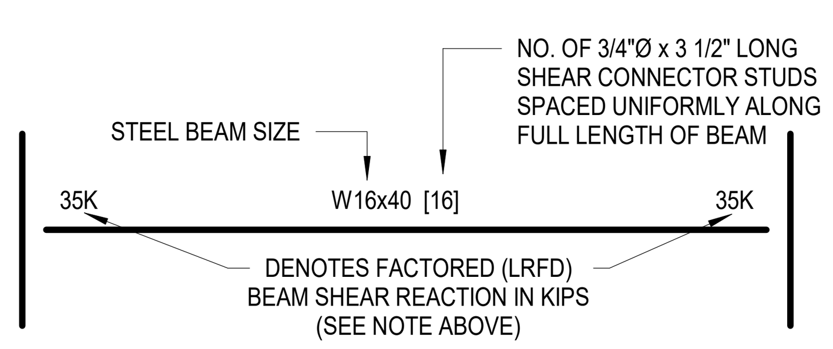

TYPICAL BEAM DIAGRAM

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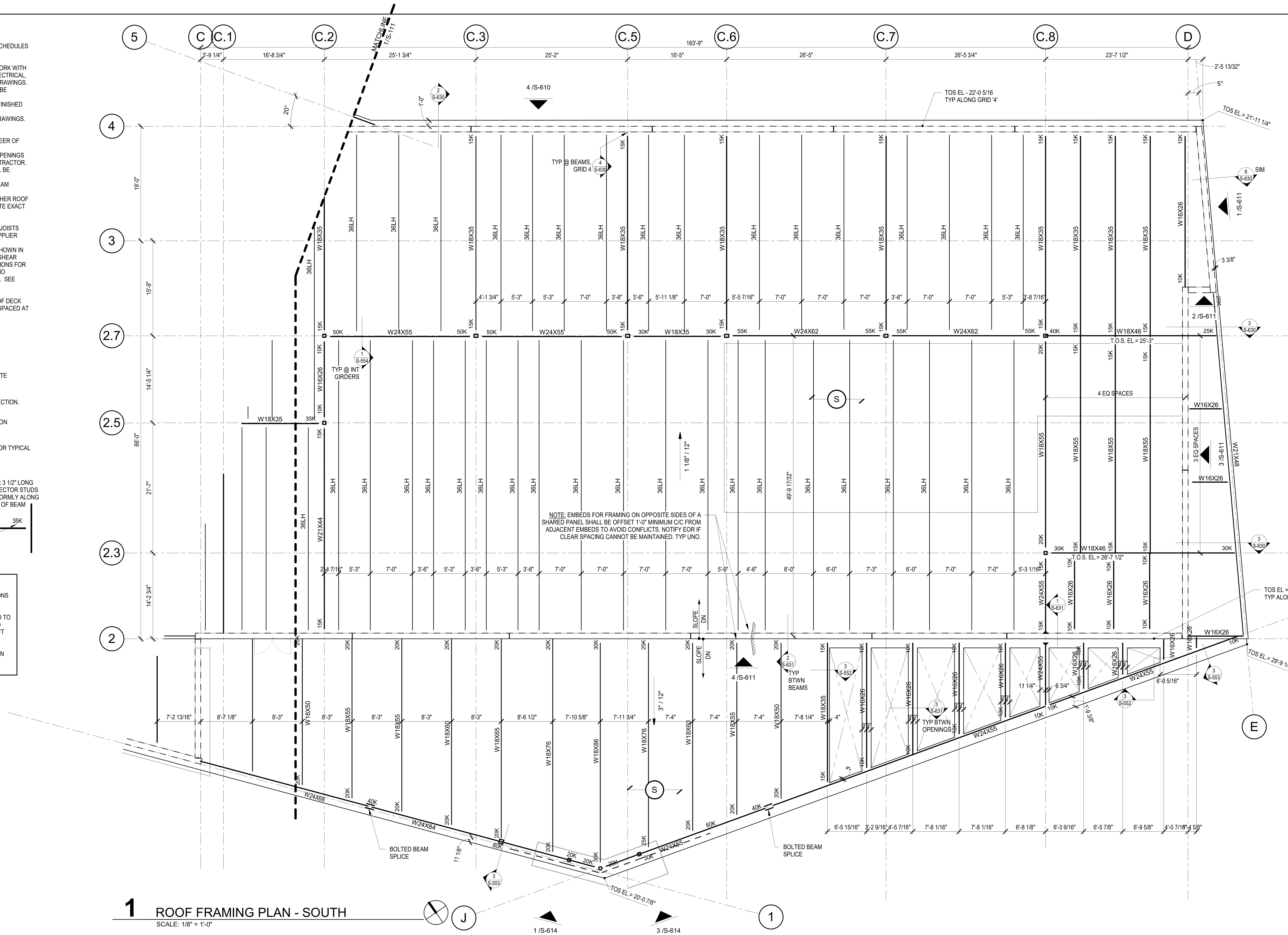

1 ROOF FRAMING PLAN - SOUTH
SCALE: 1/8" = 1'-0"

No.	Date	Issue / Revision

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SUITE 375
TAMPA, FL 33607

FLORIDA-ALABAMA TPO

ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

ROOF FRAMING PLAN - SOUTH

DWG
S-112
SHEET

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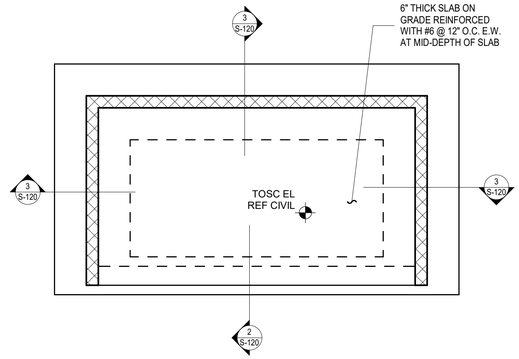
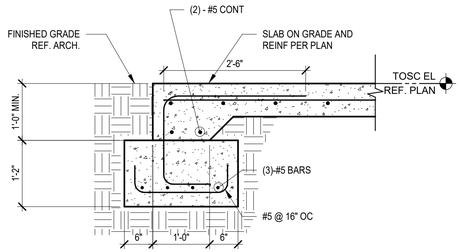


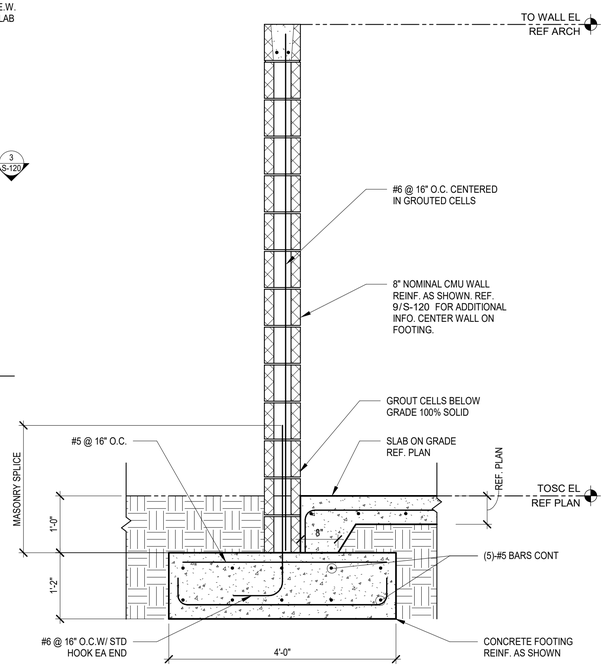
1 TRASH ENCLOSURE PLAN

SCALE: 1/4" = 1'-0"



2 SECTION

SCALE: 3/4" = 1'-0"



3 CMU TRASH ENCLOSURE FOOTING

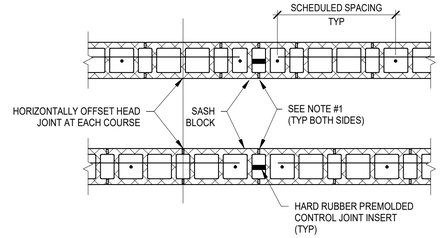
SCALE: 3/4" = 1'-0"

REINFORCEMENT SPLICE LENGTH SCHEDULE	
f'm=1500 psi MASONRY	
BAR SIZE	MASONRY
#3	1'-0"
#4	1'-3"
#5	1'-11"
#6	3'-7"
#7	5'-0"
#8	6'-0"
#9	6'-9"

NOTES:
1. LAP LENGTHS LISTED ABOVE APPLY UNDER THE FOLLOWING CONDITIONS:
   - A. REINFORCING BARS ARE GRADE 60 (Fy=60,000 PSI)
   - B. MASONRY STRENGTH IS AT LEAST 1,500 PSI
   - C. WALL BARS ARE SPACED AT LEAST 5 BAR DIA. OC
   - D. CLEAR COVER ASSUMED TO BE 2 1/2" FOR BAR SIZES OF #6 AND LESS.
   - E. CLEAR COVER ASSUMED TO BE 3" FOR #7 BARS AND 4" FOR #8 AND #9 BARS.
2. WHERE CLEAR COVER OR CLEAR SPACING FOR MASONRY REINF IS LESS, INCREASE SPLICE LENGTHS SHOWN BY MULTIPLYING LENGTHS BY MAX. RATIO OF ASSUMED CLEAR COVER TO ACTUAL CLEAR COVER.
3. WHERE A LARGER BAR LAPS A SMALLER BAR, THE LARGER SCHEDULED LAP LENGTH APPLIES UNO
4. SEE CONCRETE TYPICAL DETAILS FOR DOWELS AND HOOK LENGTHS.
5. SPLICE LENGHTS COMPLY WITH TMS 402/602-16.

4 MASONRY LAP SPLICE SCHEDULE

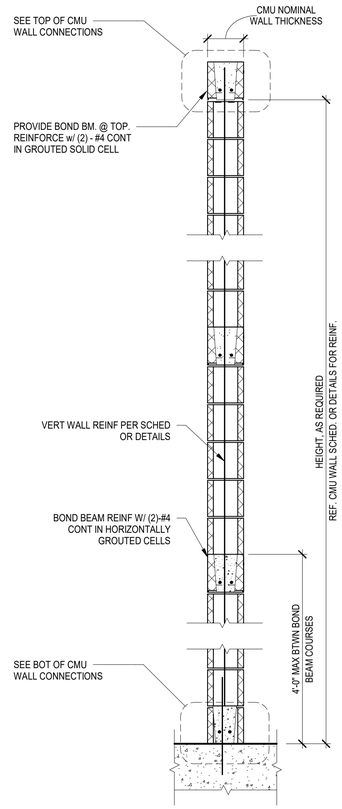
NOT TO SCALE



NOTES:
1. USE SEALANT BOTH SIDES FULL HEIGHT OF WALL.
2. MAXIMUM CONTROL JOINT SPACING IS 24'-0" OC.
3. WHERE POSSIBLE, POSITION CMU WALL CONTROL JOINT TO COINCIDE WITH SLAB-ON-GRADE CONTROL/CONSTRUCTION JOINTS.
4. SUBMIT A CMU CONTROL JOINT PLAN TO THE ARCHITECT FOR REVIEW AND APPROVAL.
5. IN LIEU OF HARD RUBBER PREMOLDED CONTROL JOINT INSERT AND SPECIAL SASH BLOCKS, A CONTROL JOINT SHEAR BAR (12" LONG x 1" WIDE x 1/4" THICK MAY BE GROUTED INTO THE WALL AT THE CONTROL JOINT AT 16" OC

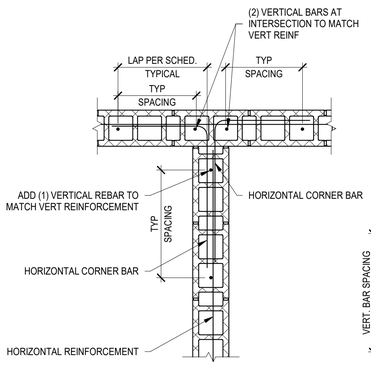
6 TYPICAL DETAIL WALL CONTROL JOINT

NOT TO SCALE

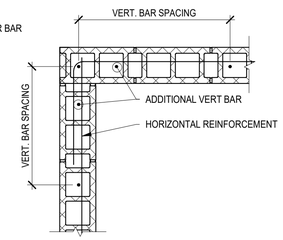


9 TYPICAL DETAIL WALL SECTION

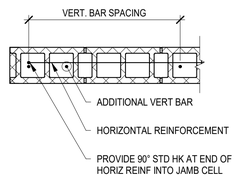
NOT TO SCALE



"T" - INTERSECTIONS



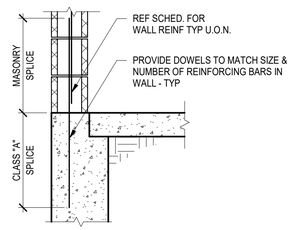
CORNER



END OR JAMB

8 TYPICAL DETAIL CMU WALL CORNER REINFORCING

NOT TO SCALE



CAST-IN-PLACE (STRAIGHT BAR)

NOTES:
1. MASONRY DOWEL TO MATCH SIZE & SPACING OF VERTICAL WALL REINF TYP
2. REF TO REINFORCEMENT SPLICE LENGTH SCHEDULE ON 4/S-120FOR ADD'L INFO

5 TYPICAL DETAIL WALL ANCHORAGE

NOT TO SCALE

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No.	Date	Issue / Revision

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FLORIDA-ALABAMA TPO

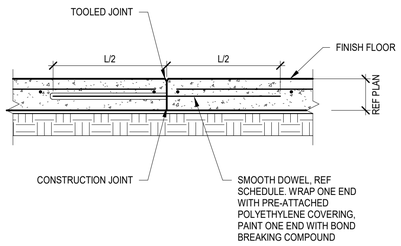
ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

ENLARGED PLAN - TRASH ENCLOSURE

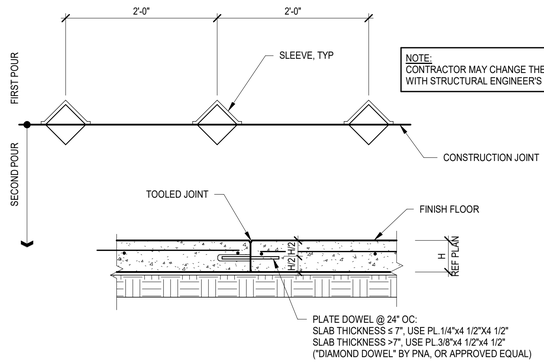
DWG
S-120
SHEET

SLAB ON GRADE SMOOTH DOWEL SCHEDULE			
THK (IN)	DOWEL Ø (IN)	L (IN)	SPACING
5-6	3/4	16	12" OC
7-8	1	18	12" OC
9-11	1 1/4	18	12" OC

NOTE:
1. CONTRACTOR MAY CHANGE THE LOCATION OF JOINT WITH STRUCTURAL ENGINEER'S PRIOR APPROVAL.
2. PROVIDE CHAIRS AS REQ'D TO PROVIDE HORIZONTAL ALIGNMENT ACROSS JOINT
3. TERMINATE MESH/ REINFORCEMENT AT CONSTRUCTION JOINT.



1 CONSTRUCTION JOINT
NOT TO SCALE

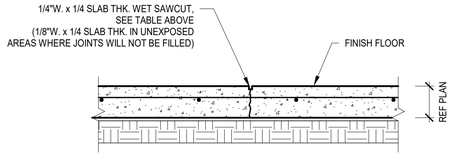


NOTE:
CONTRACTOR MAY CHANGE THE LOCATION OF JOINT WITH STRUCTURAL ENGINEER'S PRIOR APPROVAL.

2 CONSTRUCTION JOINT (DIAMOND)
NOT TO SCALE

NOTE:
CONTROL JOINTS SHALL BE SAWN NOT LATER THAN THE FOLLOWING TABLE, OR 16 HOURS AFTER FINAL FINISHING (WHICHEVER IS LESS). WHERE TEMP. EQUALS THE AMBIENT TEMPERATURE IN DEGREES FAHRENHEIT AT THE TIME OF FINAL FINISHING. SAWCUTTING SHALL BE SUSPENDED ONLY IF THE LARGE AGGREGATE IS DISLODGED OR LOOSENED. ALTERNATE: USE PRE-FORMED JOINT FORMER.

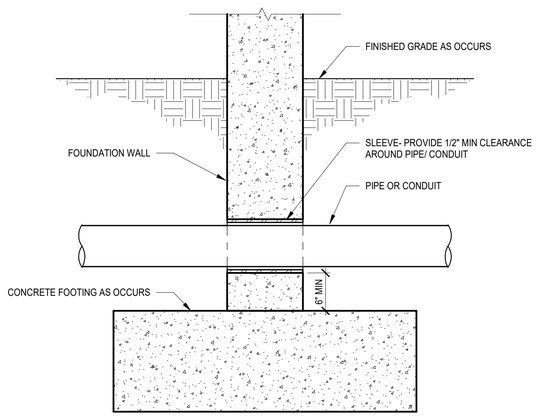
TEMP - °F	TIME - HOURS
< 40°	16
50°	14
60°	8 1/2
70°	5 1/2
80°	4
90°	3



3 CONTROL/ CONTRACTION JOINT
NOT TO SCALE

SPREAD FOOTING SCHEDULE				
FOOTING MARK	FOOTING DIMENSIONS			BARS E.W. (T&B) U.N.O.
	L	W	T	
F6	6'-0"	6'-0"	1'-6"	6-#5
F7	7'-0"	7'-0"	1'-8"	8-#5
F8	8'-0"	8'-0"	1'-10"	8-#6
F9	9'-0"	9'-0"	2'-1"	10-#6
F9.5	9'-6"	9'-6"	2'-2"	8-#7

6 SPREAD FOOTING SCHEDULE
NOT TO SCALE

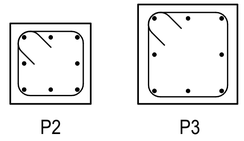


4 PIPE/ CONDUIT THROUGH FOUNDATION WALL
NOT TO SCALE

TOP & BOTTOM REINF				
FOOTING MARK	FOOTING DIMENSIONS		TOP & BOTTOM REINF	
	WIDTH	THICKNESS	LONG	TRANS
WF36	3'-0"	1'-0"	(3)-#5 BARS	#5 @ 10" OC
WF48	4'-0"	1'-0"	(4)-#5 BARS	#5 @ 10" OC

5 WALL FOOTING SCHEDULE
NOT TO SCALE

PLINTH SCHEDULE				
PLINTH MARK	SIZE		REINFORCEMENT	
	L	W	VERT BARS	TIES
P2	2'-0"	2'-0"	(8) - #6	#4 @ 9" OC



PLAN

7 PLINTH SCHEDULE AND PLAN
NOT TO SCALE

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No.	Date	Issue / Revision

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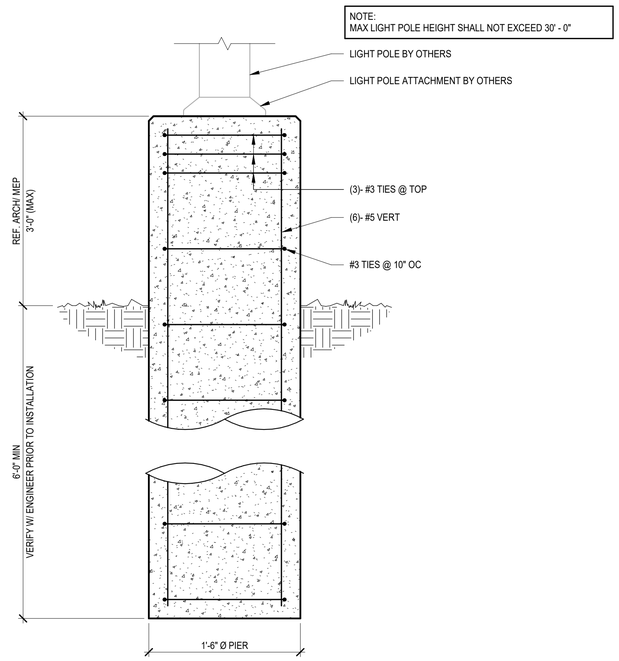
SUITE 375
TAMPA, FL 33607

FLORIDA-ALABAMA TPO		
ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

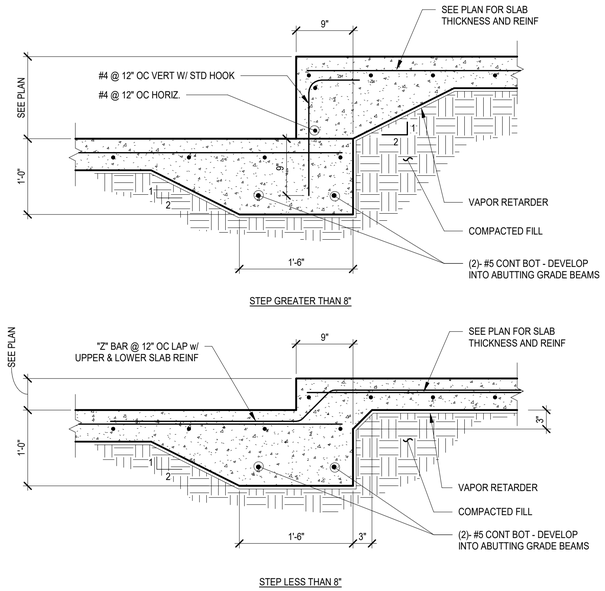
TYPICAL FOUNDATION SCHEDULE & DETAILS

DWG
S-301
SHEET



1 LIGHT POLE DETAIL

NOT TO SCALE



STEP GREATER THAN 8"

STEP LESS THAN 8"

2 STEP IN SLAB

NOT TO SCALE

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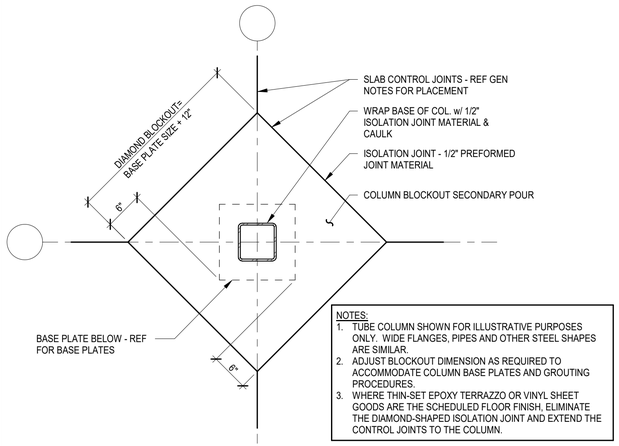
TAMPA, FL 33607

FLORIDA-ALABAMA TPO

ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

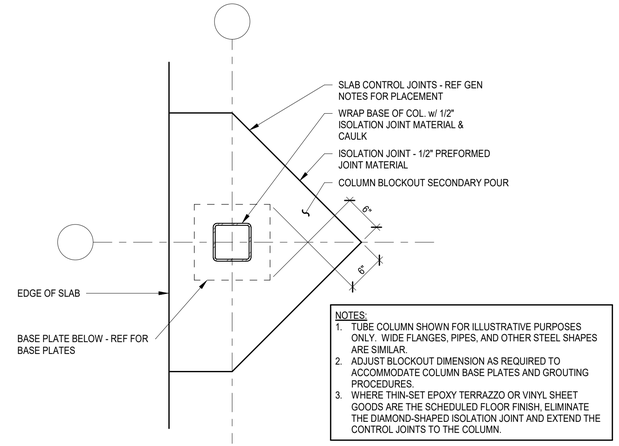
TYPICAL FOUNDATION SCHEDULE & DETAILS

DWG
S-302
SHEET



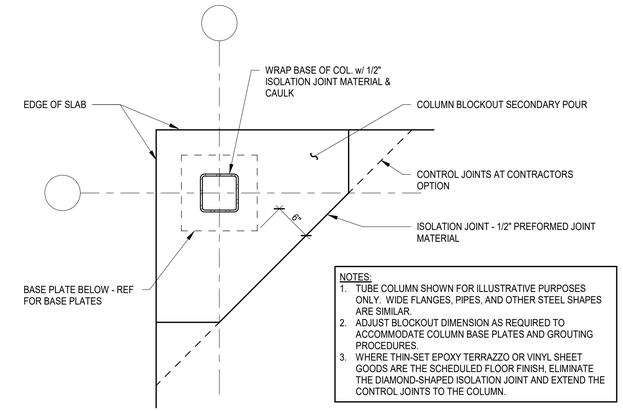
NOTES:
1. TUBE COLUMN SHOWN FOR ILLUSTRATIVE PURPOSES ONLY. WIDE FLANGES, PIPES AND OTHER STEEL SHAPES ARE SIMILAR.
2. ADJUST BLOCKOUT DIMENSION AS REQUIRED TO ACCOMMODATE COLUMN BASE PLATES AND GROUTING PROCEDURES.
3. WHERE THIN-SET EPOXY TERRAZZO OR VINYL SHEET GOODS ARE THE SCHEDULED FLOOR FINISH, ELIMINATE THE DIAMOND-SHAPED ISOLATION JOINT AND EXTEND THE CONTROL JOINTS TO THE COLUMN.

1 INTERIOR COLUMN BLOCKOUT
NOT TO SCALE



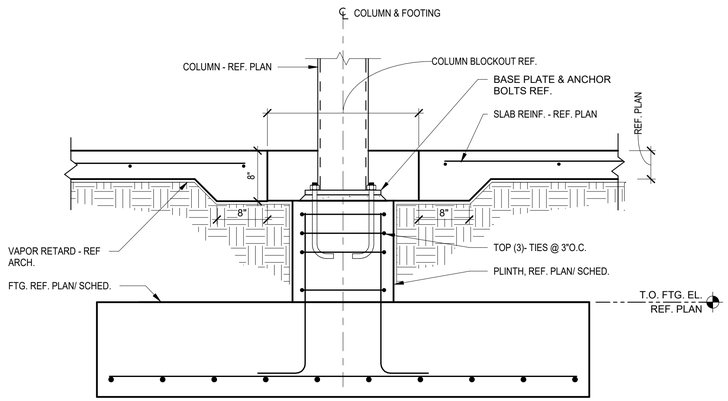
NOTES:
1. TUBE COLUMN SHOWN FOR ILLUSTRATIVE PURPOSES ONLY. WIDE FLANGES, PIPES, AND OTHER STEEL SHAPES ARE SIMILAR.
2. ADJUST BLOCKOUT DIMENSION AS REQUIRED TO ACCOMMODATE COLUMN BASE PLATES AND GROUTING PROCEDURES.
3. WHERE THIN-SET EPOXY TERRAZZO OR VINYL SHEET GOODS ARE THE SCHEDULED FLOOR FINISH, ELIMINATE THE DIAMOND-SHAPED ISOLATION JOINT AND EXTEND THE CONTROL JOINTS TO THE COLUMN.

2 EXTERIOR COLUMN BLOCKOUT
NOT TO SCALE

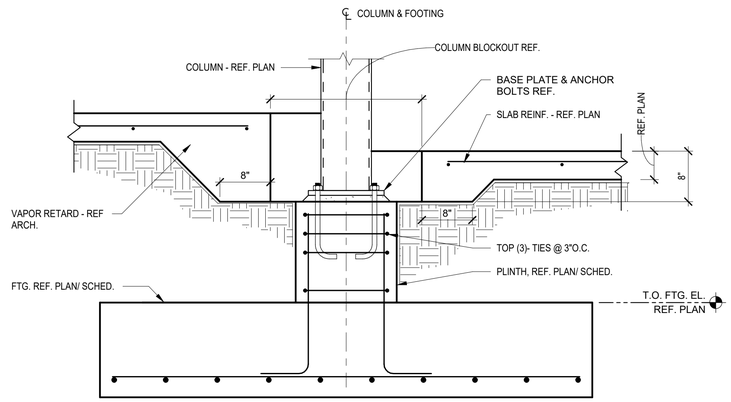


NOTES:
1. TUBE COLUMN SHOWN FOR ILLUSTRATIVE PURPOSES ONLY. WIDE FLANGES, PIPES, AND OTHER STEEL SHAPES ARE SIMILAR.
2. ADJUST BLOCKOUT DIMENSION AS REQUIRED TO ACCOMMODATE COLUMN BASE PLATES AND GROUTING PROCEDURES.
3. WHERE THIN-SET EPOXY TERRAZZO OR VINYL SHEET GOODS ARE THE SCHEDULED FLOOR FINISH, ELIMINATE THE DIAMOND-SHAPED ISOLATION JOINT AND EXTEND THE CONTROL JOINTS TO THE COLUMN.

3 EXTERIOR COLUMN BLOCKOUT
NOT TO SCALE



4 INTERIOR COLUMN DETAIL AT SPREAD FOOTING
NOT TO SCALE



5 INTERIOR COLUMN DETAIL AT SPREAD FOOTING AT DROP
NOT TO SCALE

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TAMPA, FL 33607

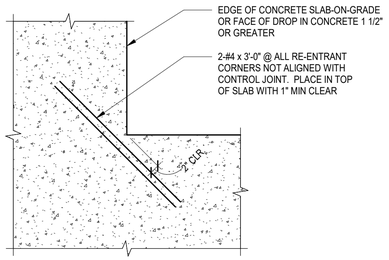
FLORIDA-ALABAMA TPO

ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

TYPICAL FOUNDATION DETAILS

DWG S-303

SHEET



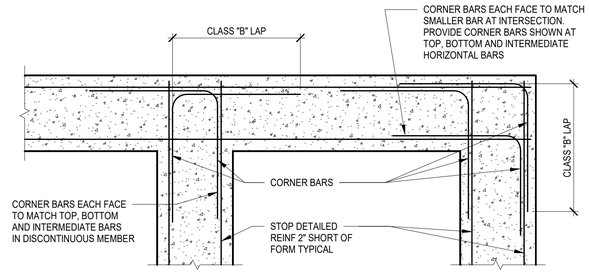
PLAN

1 TYPICAL SLAB-ON-GRADE RE-ENTRANT CORNER REINFORCING DETAIL

NOT TO SCALE

NOTES:

1. MATCH SIZE, LOCATION AND NUMBER OF HORIZONTAL BEAM AND WALL BARS, EXCEPT THAT WHERE THERE ARE MORE THAN 2 TOP OR BOTTOM BARS, ONLY THE INSIDE AND OUTSIDE BARS MUST BE MATCHED.
2. WHERE 90 DEGREE HOOKS ARE PROVIDED FOR TOP BARS CORNER BARS MAY BE OMITTED AT TOP. WHERE 90 DEGREE HOOKS ARE PROVIDED FOR BOTTOM BARS, CORNER BARS MAY BE OMITTED AT BOTTOM.



PLAN

2 TYPICAL CORNER BARS AT STRIP FOOTING INTERSECTION DETAIL

NOT TO SCALE

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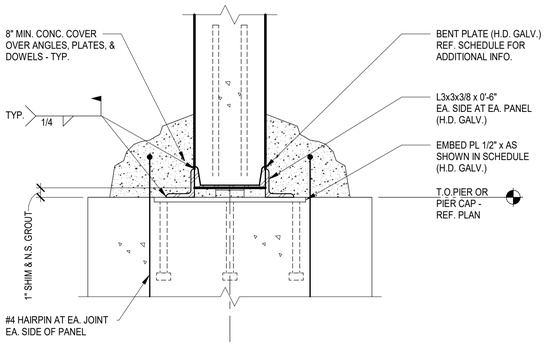
FLORIDA-ALABAMA TPO

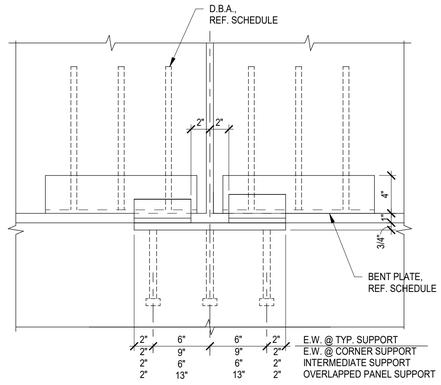
ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

TYPICAL FOUNDATION DETAILS

DWG
S-304
SHEET



SECTION

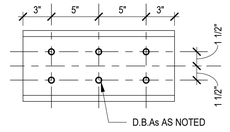


ELEVATION - TYPICAL

EMBED PLATE SCHED.		
LOCATION	PLATE SIZE	NO. H.S.A.
TYPICAL	1/2" x 16"x 1'-4"	9
CORNER	1/2" x 22" x 1'-10"	9
INTERMEDIATE	1/2" x 12" x 1'-4"	6
OVERLAPPED	1/2" x 28" x 2'-4"	9

NOTES:

1. PROVIDE 2-1"Ø AIR RELIEF HOLES IN CENTER OF PLATE.
2. H.S.A. ARE 3/4" DIA. x 0'-8".
3. H.D. GALV. AFTER FABRICATION.



BENT PLATE PLAN

BENT PLATE SCHEDULE		
PANEL THK.	PLATE SIZE	
11 1/4"	PL5/8" X AS SHOWN	(6)- 5/8"Ø D.B.As x 2'-0" SPACED AS SHOWN

NOTE: D.B.As SHALL NOT BE SUBSTITUTED BY REBAR

1 TYPICAL TILT-UP WALL PANEL ANCHOR DETAIL

NOT TO SCALE

No.	Date	Issue / Revision

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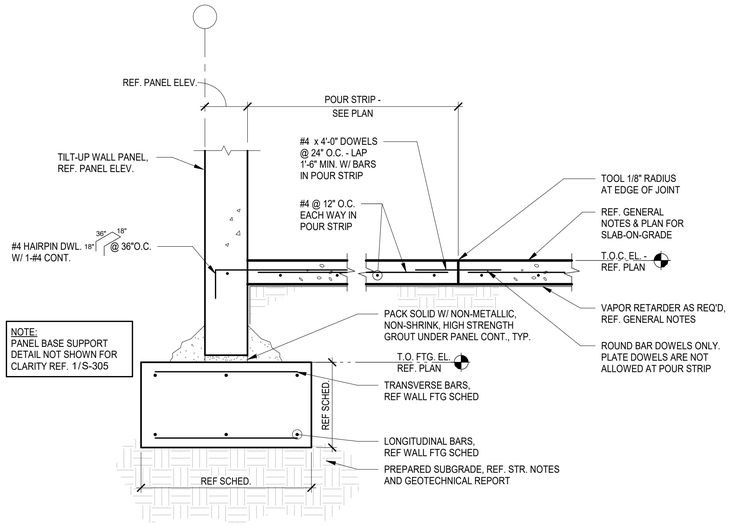
FLORIDA-ALABAMA TPO

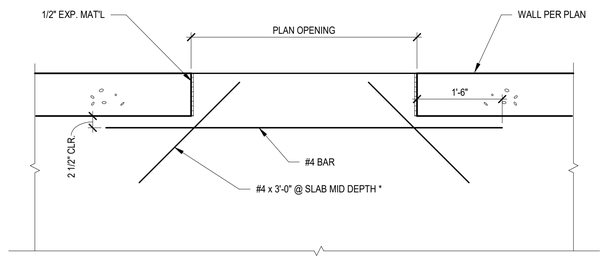
ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

TYPICAL TILT WALL DETAILS - FOUNDATION

DWG S-305 SHEET

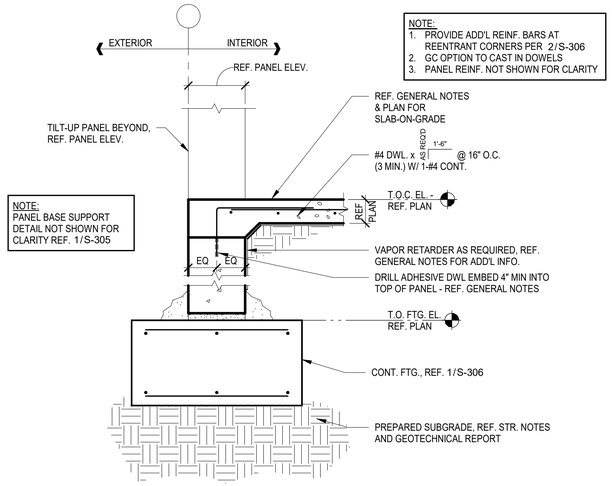


1 SLAB POUR STRIP DETAIL
NOT TO SCALE



*TYP. SLAB REINF. @ ALL REENTRANT CORNERS

2 TYPICAL SLAB OPENING PLAN DETAIL
NOT TO SCALE



3 TILT-UP WALL PANEL AT MAN DOOR DETAIL
NOT TO SCALE

4 TYPICAL TILT-UP WALL PANEL TO CONT. FOOTING CONNECTION DETAIL
NOT TO SCALE

5 TILT-UP WALL PANEL TO CONT. FOOTING CONNECTION DETAIL - CORNER CONDITION
NOT TO SCALE

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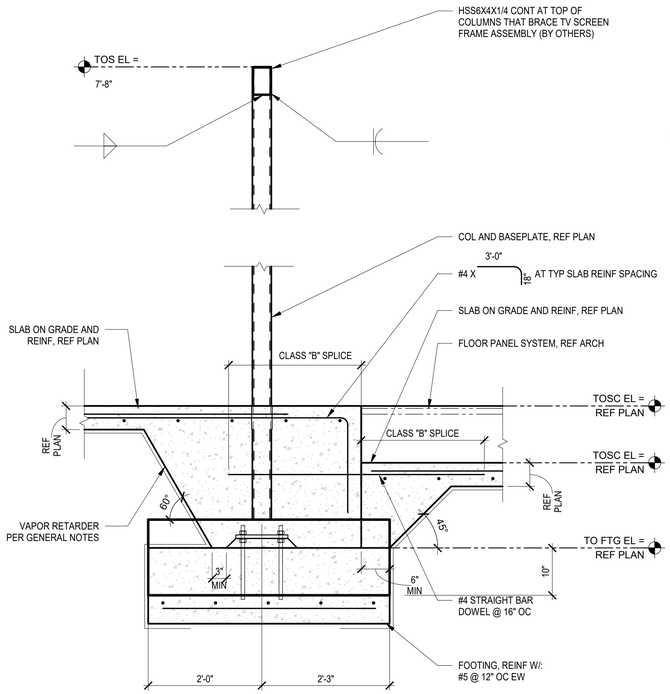
FLORIDA-ALABAMA TPO

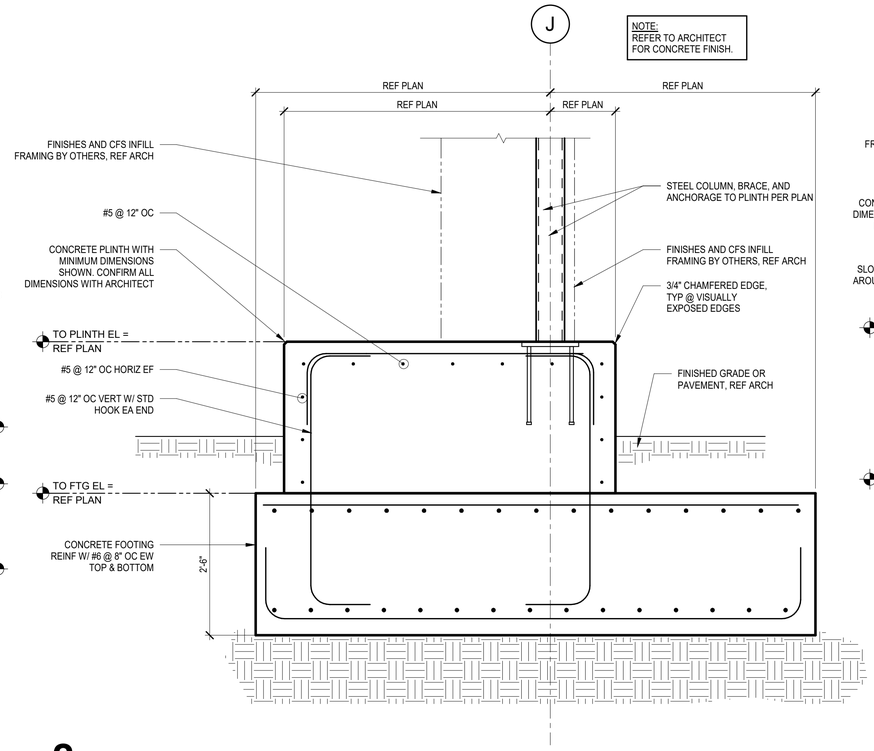
ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

TYPICAL TILT WALL DETAILS - FOUNDATION

DWG S-306
SHEET



1 SECTION
NOT TO SCALE



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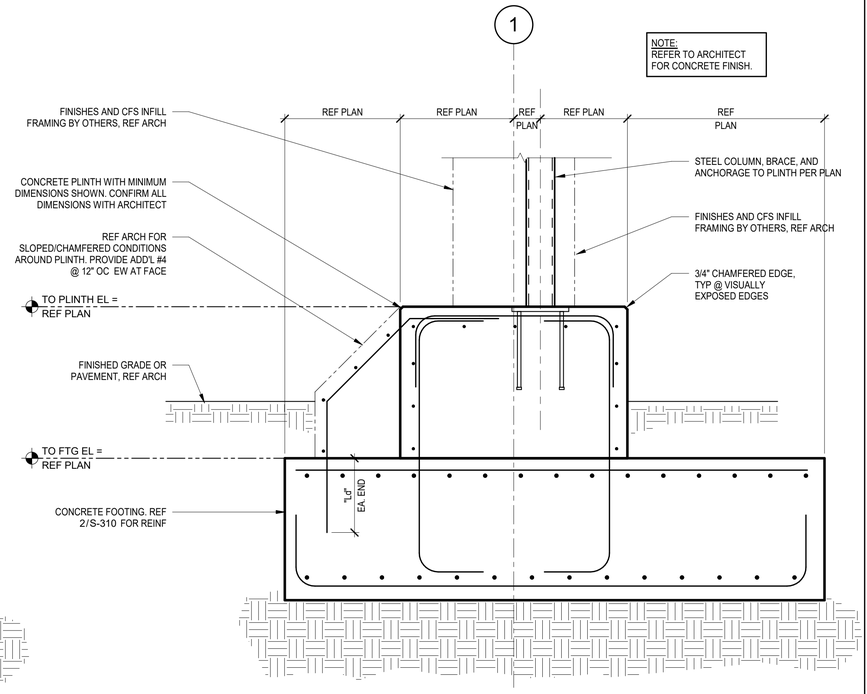

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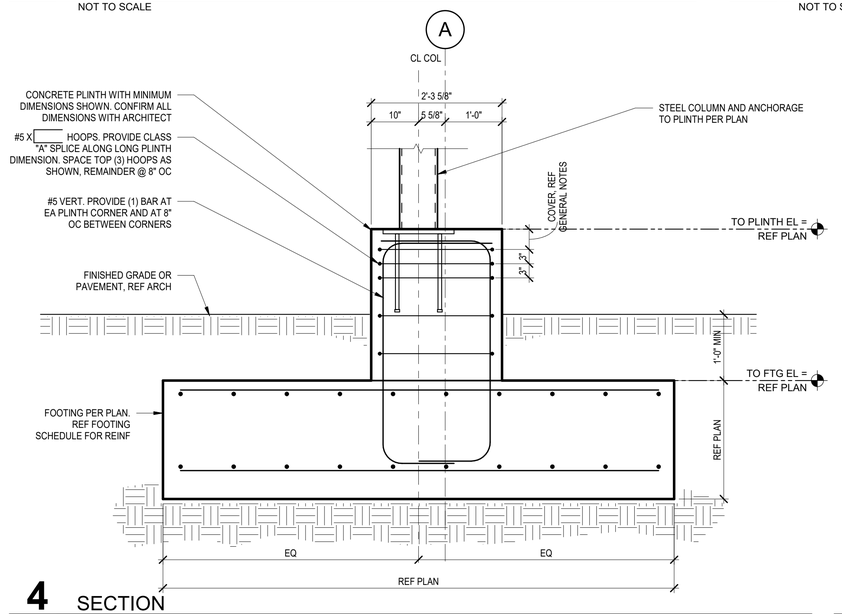

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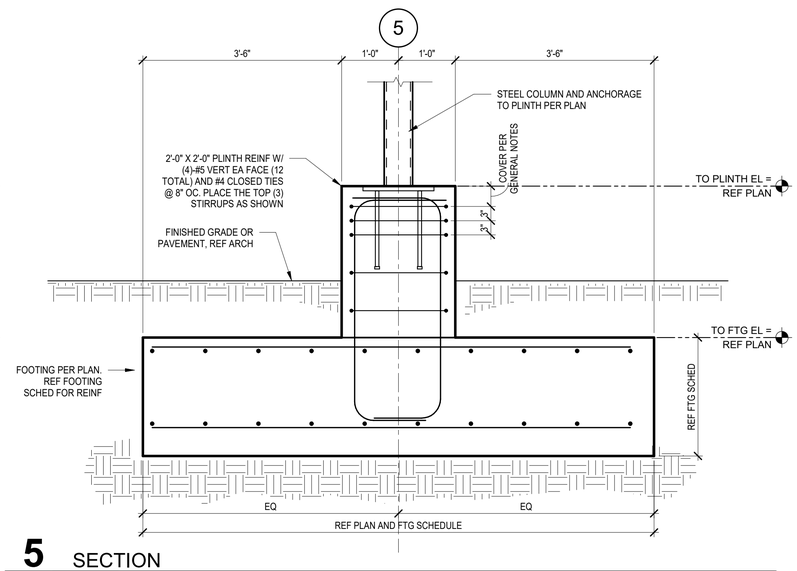

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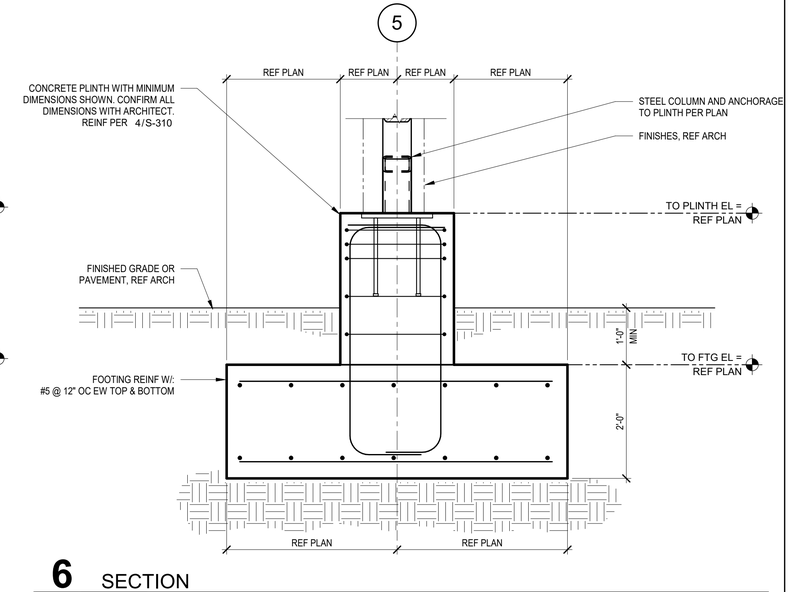

6 SECTION
NOT TO SCALE

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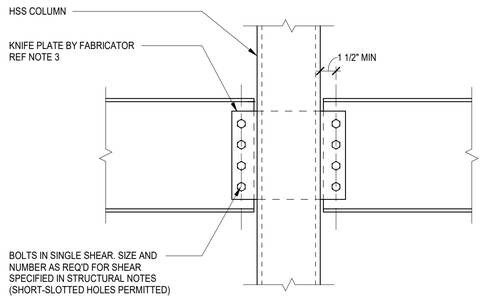
TAMPA, FL 33607

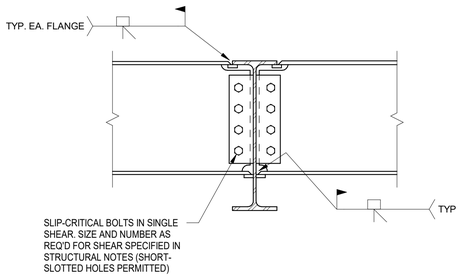
FLORIDA-ALABAMA TPO

ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

FOUNDATION DETAILS

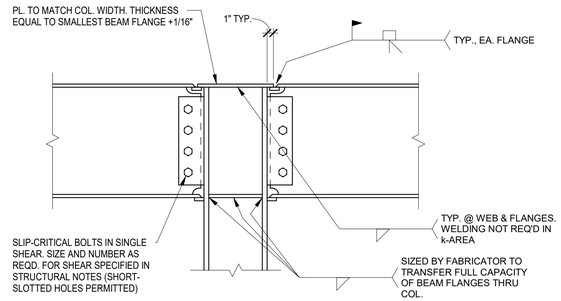
DWG S-310
SHEET





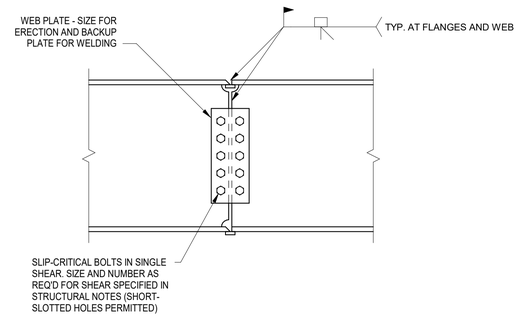
2 BEAM TO BEAM MOMENT CONNECTION TYPICAL DETAIL

NOT TO SCALE



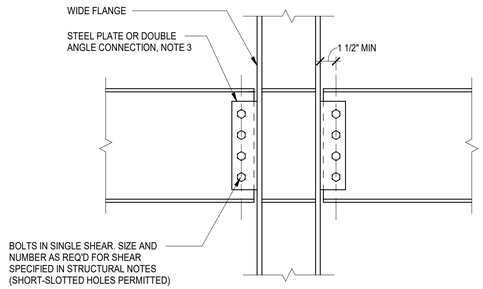
3 BEAM TO TOP OF COLUMN MOMENT CONNECTION TYPICAL DETAIL

NOT TO SCALE

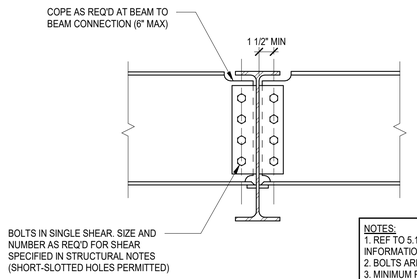


4 MOMENT CONNECTION (BEAM SPLICE) TYPICAL DETAIL

NOT TO SCALE



BEAM SIZE	MINIMUM NO. OF ROWS OF BOLTS (N)
W8	2
W10	2
W12	2
W14	3
W16	3
W18	4
W21	4
W24	5
W27	5
W30	6
W33	7
W36	8
W40	9
W44	10



NOTES:
1. REF TO 5.1-D OF GENERAL NOTES FOR INFORMATION NOT SHOWN.
2. BOLTS ARE A325N, TYPICAL
3. MINIMUM PLATE THICKNESS IS 3/8" TYPICAL AND 7/16" AT W33 AND DEEPER. MINIMUM ANGLE THICKNESS IS 1/4" TYPICAL AND 5/16" AT W33 AND DEEPER.

1 SHEAR CONNECTION TYPICAL DETAIL

NOT TO SCALE

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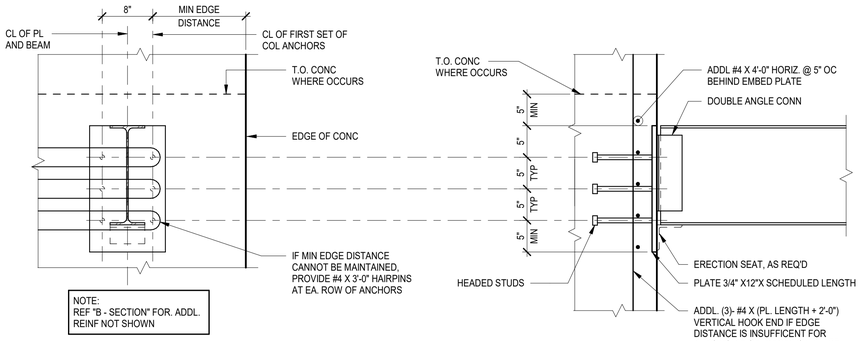
FLORIDA-ALABAMA TPO

ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

STANDARD BEAM CONNECTION DETAILS

DWG
S-550
SHEET

MAX. FACTORED END REACTION (KIPS)	NUMBER OF STUDS	PLATE LENGTH (NOTE 4)	MIN EDGE DISTANCE
35K	4	1'-3"	12"
55K	6	1'-8"	2'-0"
70K	8	2'-1"	2'-6"
85K	10	2'-6"	3'-0"
100K	12	2'-11"	3'-6"



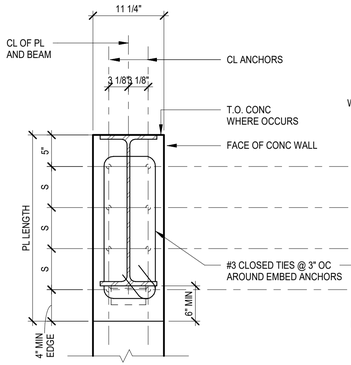
A ELEVATION

B SECTION

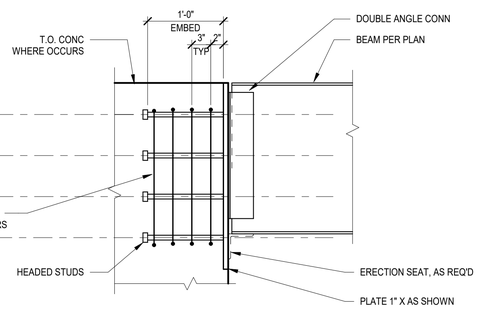
NOTES:
1. PROVIDE EMBED PLATE TO EQUAL OR EXCEED BEAM REACTION AS SPECIFIED FOR BEAM CONNECTION DESIGN.
2. DOUBLE ANGLE CONNECTION SHALL BE DESIGNED AND DETAILED BY A PROFESSIONAL ENGINEER WORKING UNDER THE GUIDANCE OF THE CONTRACTOR. REF GENERAL NOTES UNDER "STRUCTURAL STEEL CONNECTIONS."
3. HEADED STUDS SHALL BE 3/4"Ø X 6".
4. PROVIDE EXTRA PLATE LENGTH AS REQUIRED FOR ERECTION SEAT AT CONTRACTOR'S OPTION.
5. CONTRACTOR RESPONSIBLE FOR COORDINATING CONFLICTS WITH ARCH.
6. MINIMUM CONCRETE THICKNESS SHALL BE 10".
7. FOR REACTIONS GREATER THAN 100K, REF PLAN FOR CONNECTION DETAIL.

1 STEEL BEAM TO CONCRETE WALL CONNECTION TYPICAL DETAIL (12" THICK MAX)

NOT TO SCALE



A ELEVATION

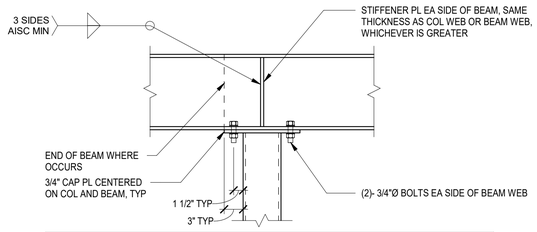


B SECTION

NOTE:
1. ANCHOR ROW SPACING "S" SHALL BE 6-1/2", UNLESS NOTED OTHERWISE.
2. PROVIDE NUMBER OF ANCHOR ROWS AS REQUIRED TO FILL THE PLATE LENGTH AT SPACING "S" AND MAINTAIN INDICATED EDGE DISTANCE.
3. DOUBLE ANGLE CONNECTION SHALL BE DESIGNED AND DETAILED BY A PROFESSIONAL ENGINEER WORKING UNDER THE GUIDANCE OF THE CONTRACTOR. REF GENERAL NOTES UNDER "STRUCTURAL STEEL CONNECTIONS."
4. HEADED STUDS SHALL BE 3/4"Ø X AS SHOWN.
5. PROVIDE EXTRA PLATE LENGTH AS REQUIRED FOR ERECTION SEAT AT CONTRACTOR'S OPTION.

6 STEEL BEAM CONNX TO END OF PANEL

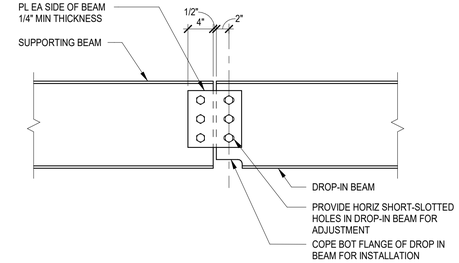
NOT TO SCALE



NOTES:
1. REFER TO ROOF PLAN FOR ROOF SLOPE. SLOPE CAP PLATES ACCORDINGLY.
2. CONNECT INTERSECTING BEAMS TO STIFFENER PLATES USING BOLTS IN SINGLE SHEAR DESIGNED FOR ECCENTRIC BEAM REACTION.
3. FOR BEAM FLANGE WIDTH <5", FIELD WELD BEAM TO CAP IN LIEU OF BOLTING, OR EXTEND BOTTOM FLANGE WITH PLATE EA. SIDE. PLATE EXTENSIONS SHALL BE SAME THICKNESS AS FLANGE, SAME LENGTH AS CAP PL., AND WELDED TO FLANGE WITH PREQUALIFIED FULL PEN. WELDS.

2 HSS OR PIPE COLUMN CAP PLATE TO BEAM CONNECTION TYPICAL DETAIL

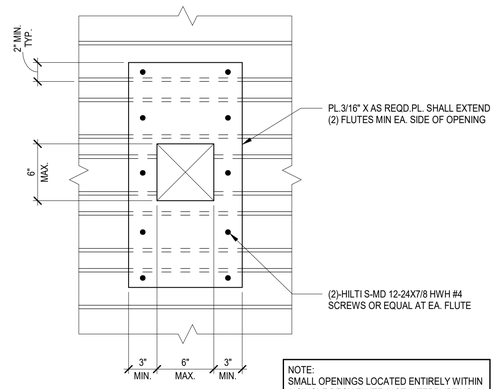
NOT TO SCALE



NOTES:
1. SPLICE CONNECTIONS SHALL BE DESIGNED BY THE CONTRACTOR TO MEET THE REQUIREMENTS SPECIFIED FOR STANDARD CONNECTIONS IN THE STRUCTURAL NOTES.
2. PROVIDE SHIM PLATES AS REQUIRED.

3 BOLTED BEAM SPLICE TYPICAL DETAIL

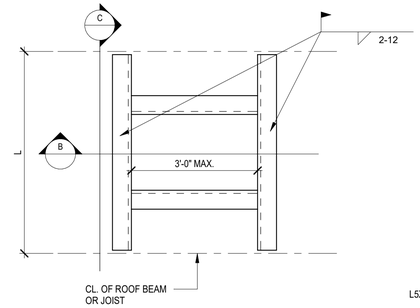
NOT TO SCALE



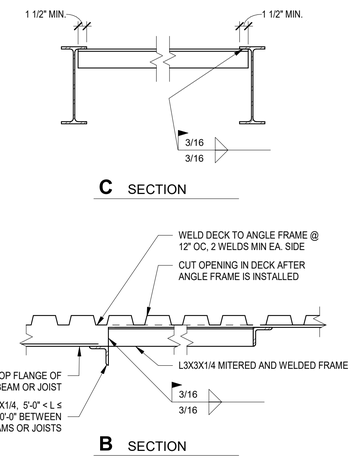
NOTE:
SMALL OPENINGS LOCATED ENTIRELY WITHIN A SINGLE DECK FLUTE (NOT INTERRUPTING A DECK VERTICAL) DO NOT REQUIRE REINFORCEMENT.

4 SMALL OPENING IN ROOF DECK

NOT TO SCALE



A PLAN



B SECTION

5 ROOF OPENING TYPICAL DETAIL

NOT TO SCALE

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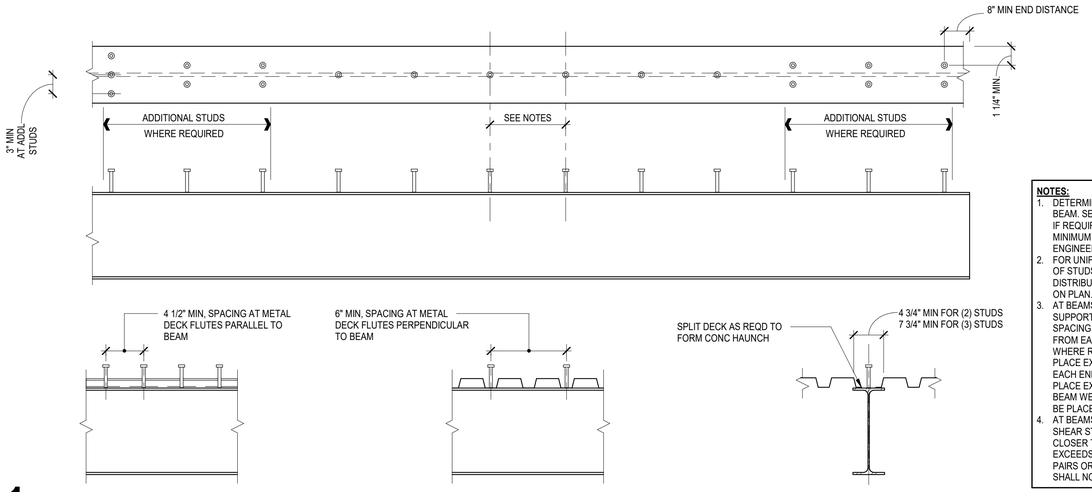
FLORIDA-ALABAMA TPO

ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

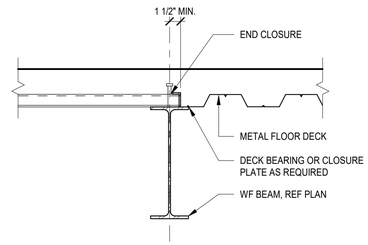
TYPICAL STEEL DETAILS

DWG
S-551
SHEET

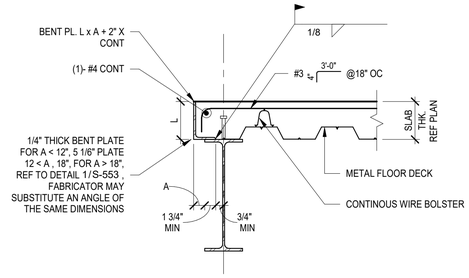


NOTES:
1. DETERMINE WHERE ALL STUDS ARE TO BE PLACED BEFORE WELDING TO BEAM. SEE DETAIL FOR STUD SPACING AND EDGE DISTANCE REQUIREMENTS. IF REQUIRED NUMBER OF STUDS CANNOT BE INSTALLED ON BEAM WITHIN MINIMUM SPACING REQUIREMENTS, GENERAL CONTRACTOR SHALL CONTACT ENGINEER OF RECORD FOR DIRECTION PRIOR TO INSTALLING ANY STUDS.
2. FOR UNIFORMLY-DISTRIBUTED STUDS - PLACE HALF OF THE TOTAL NUMBER OF STUDS EACH SIDE OF THE BEAM MIDSPAN. FOR NON-UNIFORMLY-DISTRIBUTED STUDS - PLACE NUMBER STUDS BETWEEN BEAMS AS NOTED ON PLAN.
3. AT BEAMS PERPENDICULAR TO DECK FLUTES: STARTING FROM THE END SUPPORT PLACE SHEAR STUDS UNIFORMLY TOWARDS THE MIDSPAN. MAX SPACING SHALL NOT EXCEED 18 INCHES. FOR REMAINING STUDS, BEGIN FROM EACH END AND PLACE STUDS AT REMAINING FLUTES WITHOUT STUDS. WHERE REQUIRED NUMBER OF STUDS EXCEEDS SINGLE STUDS PER FLUTE, PLACE EXTRA STUDS IN PAIRS (1-1/2" EACH SIDE OF BEAM WEB), STARTING AT EACH END OF BEAM. WHERE REQUIRED NUMBER OF STUDS EXCEEDS PAIRS, PLACE EXTRA STUDS IN TRIOS (ONE AT CENTER, PLUS ONE 3" EACH SIDE OF BEAM WEB), STARTING AT EACH END OF BEAM. MULTIPLE STUDS SHALL NOT BE PLACED IN A LONGITUDINAL LINE WITHIN A SINGLE DECK FLUTE.
4. AT BEAMS PARALLEL TO DECK FLUTES: STARTING FROM EACH END, PLACE SHEAR STUDS EVENLY-SPACED ALONG EACH SECTION OF BEAM. SPACE NO CLOSER THAN 4 1/2" ON CENTER. WHERE REQUIRED NUMBER OF STUDS EXCEEDS SINGLE LINE SPACED 4 1/2" ON CENTER, PLACE EXTRA STUDS IN PAIRS OR TRIOS AS DESCRIBED IN PREVIOUS NOTE. MAX. SPACING OF STUDS SHALL NOT EXCEED 18 INCHES.

1 SHEAR STUD PLACEMENT DIAGRAM
NOT TO SCALE

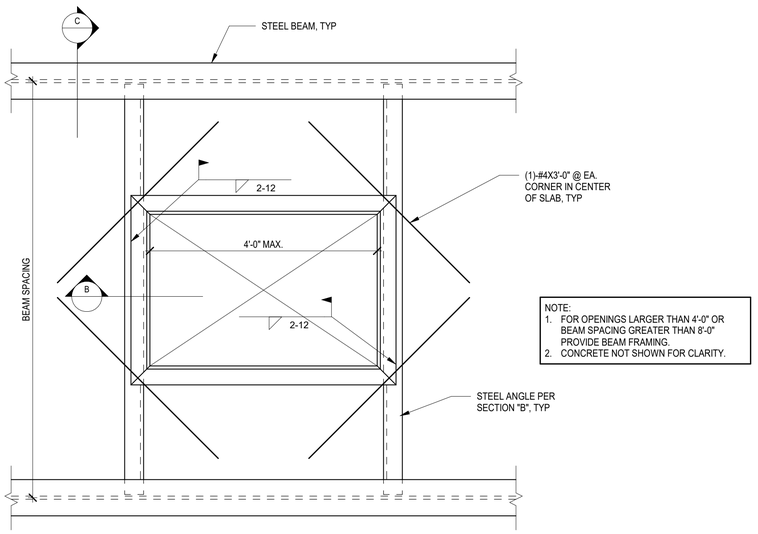


2 CHANGE OF DECK DIRECTION OVER BEAM TYPICAL DETAIL
NOT TO SCALE



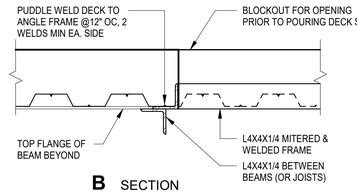
NOTE:
1. PROVIDE 1/2" RETURN LIP ON 16 GA. EDGE FORMS.

3 COMPOSITE SLAB EDGE CONDITION TYPICAL DETAIL
NOT TO SCALE

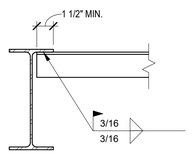


NOTE:
1. FOR OPENINGS LARGER THAN 4'-0" OR BEAM SPACING GREATER THAN 8'-0" PROVIDE BEAM FRAMING.
2. CONCRETE NOT SHOWN FOR CLARITY.

A PLAN

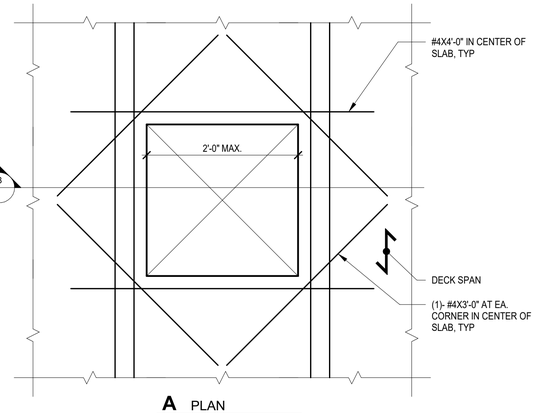


B SECTION



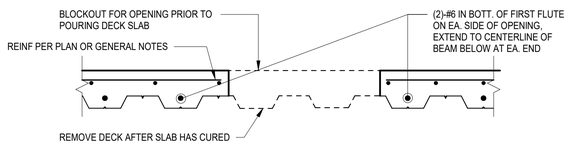
C SECTION

4 LARGE OPENING IN COMPOSITE SLAB TYPICAL DETAIL
NOT TO SCALE



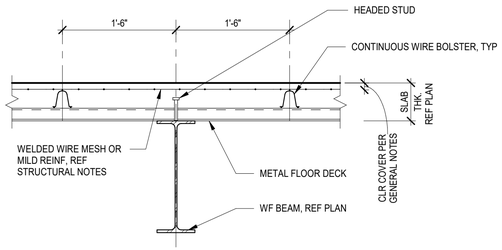
A PLAN

NOTES:
1. REINFORCING NOT REQUIRED AT OPENINGS LESS THAN 6" WIDE UNLESS DETAILED OTHERWISE.
2. FOR OPENINGS LARGER THAN 2'-0" WIDE, REF 4/S-552

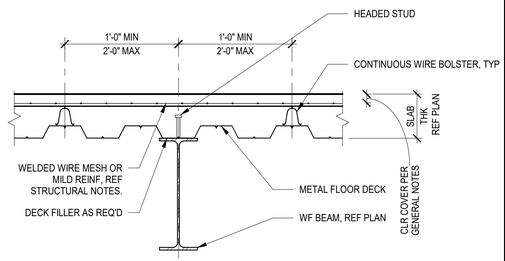


B SECTION

5 SMALL OPENING IN COMPOSITE SLAB TYPICAL DETAIL
NOT TO SCALE



A BEAM SECTION PERPENDICULAR TO DECK SPAN



B GIRDER SECTION PARALLEL TO DECK SPAN

6 COMPOSITE BEAM OR GIRDER TYPICAL DETAIL
NOT TO SCALE

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FLORIDA-ALABAMA TPO		
ROAD	COUNTY	FINANCIAL PROJECT
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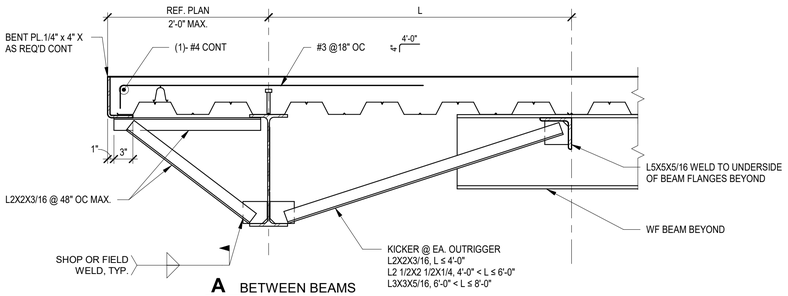
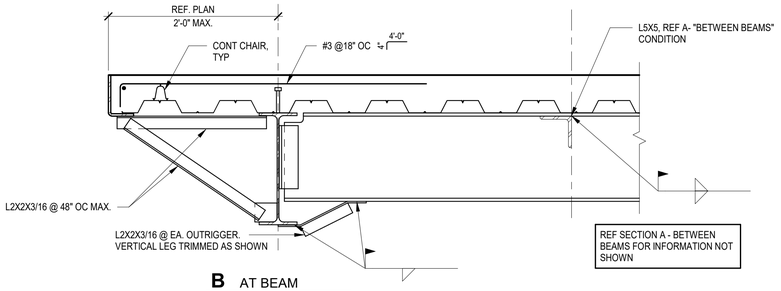
STANDARD COMPOSITE DECK DETAILS

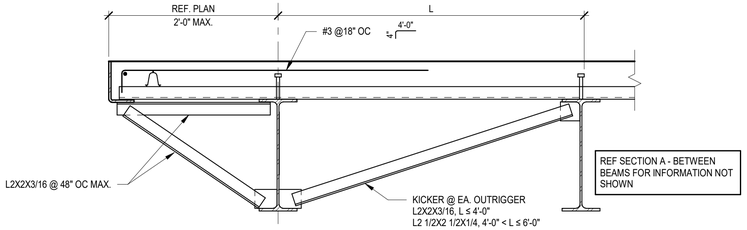
DWG
S-552
SHEET



A BETWEEN BEAMS



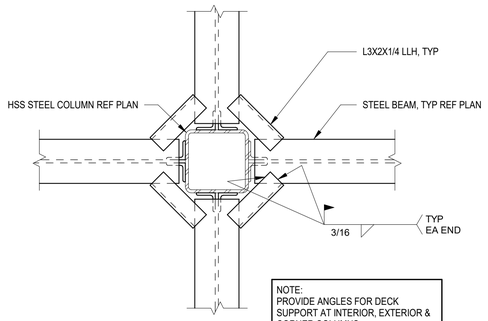
B AT BEAM



C DECK PERPENDICULAR TO BEAMS

1 COMPOSITE SLAB EDGE OVERHANG TYPICAL DETAIL

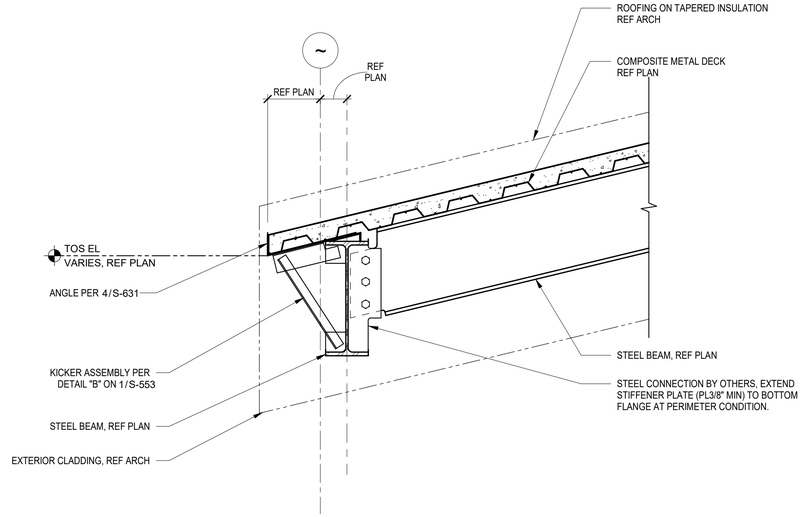
NOT TO SCALE



A PLAN

2 DECK SUPPORT AT HSS COLUMN TYPICAL DETAIL

NOT TO SCALE



3 SECTION

SCALE: 3/4" = 1'-0"

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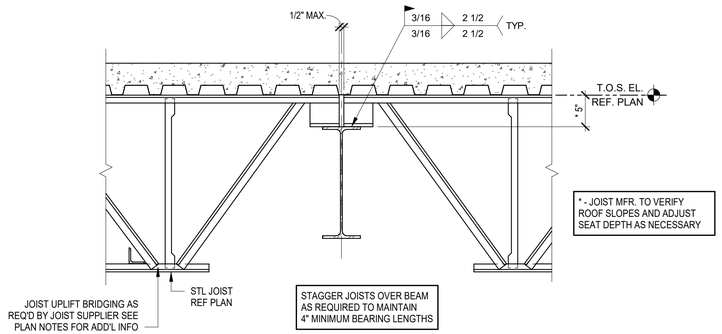
ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

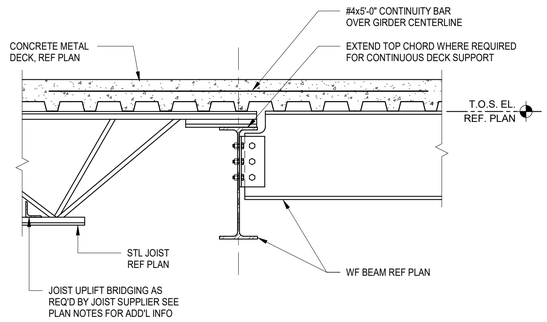
STANDARD COMPOSITE DECK DETAILS

DWG
S-553
SHEET



1 LH SERIES STEEL JOIST
BEARING ON STEEL BEAM TYPICAL DETAIL

NOT TO SCALE



2 EXTENDED TOP CHORD
FOR DECK SUPPORT TYPICAL DETAIL

NOT TO SCALE

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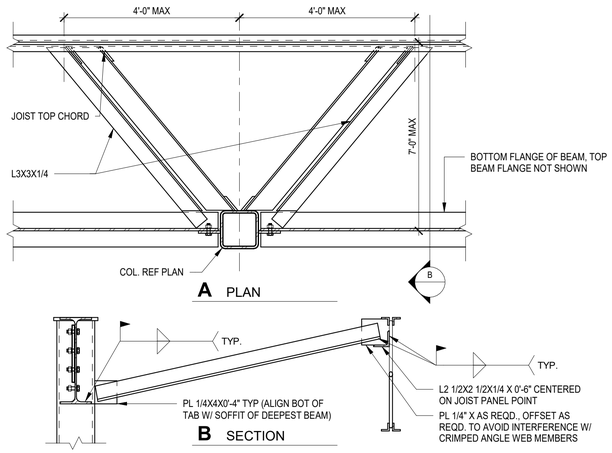
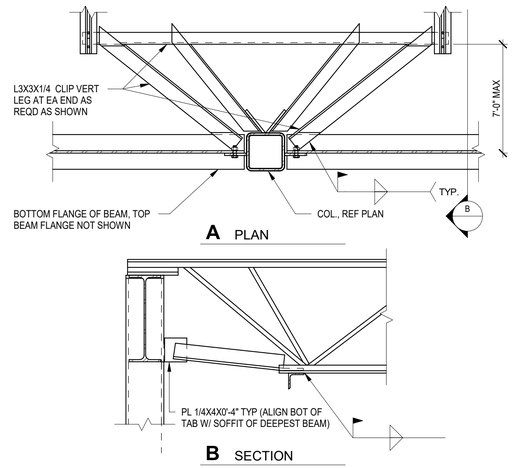
STANDARD STEEL JOISTS DETAILS

DWG
S-554
SHEET



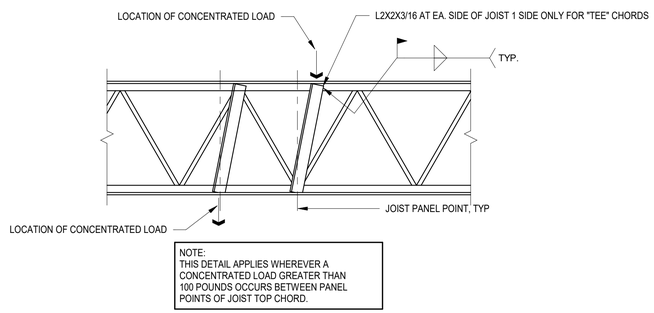
1 COLUMN BRACED TO JOISTS PARALLEL TO BEAM-TYPICAL DETAIL

NOT TO SCALE



2 COLUMN BRACED TO JOISTS PERPENDICULAR TO BEAM-TYPICAL DETAIL

NOT TO SCALE



3 JOIST CHORD REINFORCEMENT FOR CONCENTRATED POINT LOADS TYPICAL DETAIL

NOT TO SCALE

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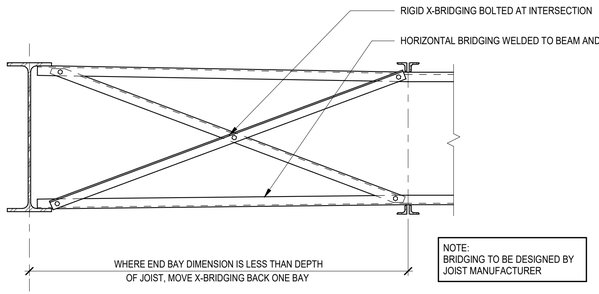
STANDARD STEEL JOISTS DETAILS

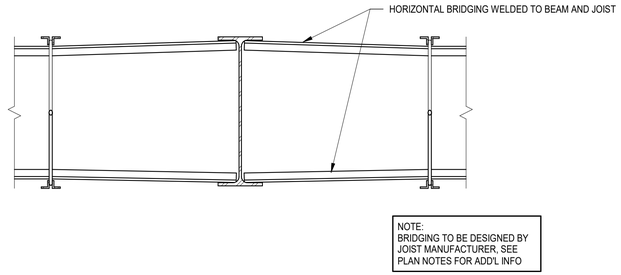
DWG S-555

SHEET



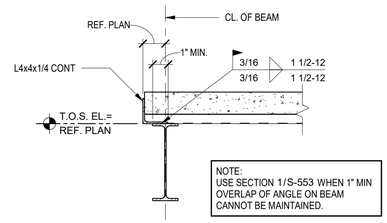
1 CROSS BRIDGING AT END BAY FOR K & KCS SERIES JOISTS TYPICAL DETAIL

NOT TO SCALE



2 HORIZONTAL BRIDGING AT BEAM PARALLEL TO JOISTS TYPICAL DETAIL

NOT TO SCALE



3 EXTERIOR BEAM PARALLEL TO JOIST TYPICAL DETAIL

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FLORIDA-ALABAMA TPO

ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

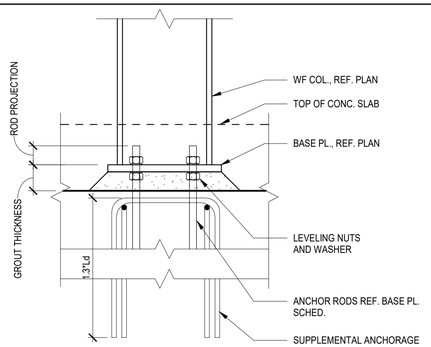
STANDARD STEEL JOISTS DETAILS

DWG
S-556
SHEET

ANCHOR ROD HOLES IN BASE PLATES			
ANCHOR ROD DIAMETER	HOLE DIAMETER	MIN. WASHER SIZE	MIN. WASHER THICKNESS
3/4"	1 5/16"	2"	1/4"
7/8"	1 9/16"	2 1/2"	5 /16"
1"	1 13/16"	3"	3/8"
1 1/4"	2 1/16"	3"	1/2"
1 1/2"	2 5/16"	3 1/2"	1/2"
1 3/4"	2 3/4"	4"	5/8"
2"	3 1/4"	5"	3/4"
2 1/2"	3 3/4"	5 1/2"	7/8"

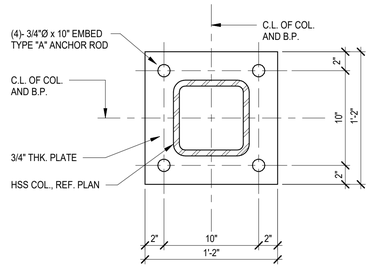
NOTES:
i. CIRCULAR OR SQUARE WASHERS MEETING THE WASHER SIZE ARE ACCEPTABLE.
ii. WASHERS SHALL BE FIELD WELDED ALL AROUND TO BASE PL. USING MINIMUM WELDS SIZE PER AISC.

1 BASE PLATE SCHEDULE
NOT TO SCALE



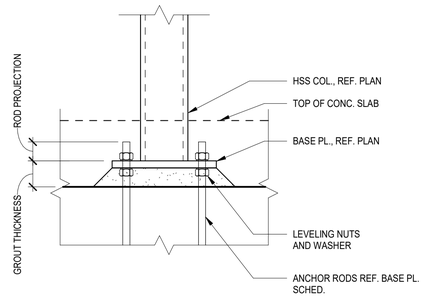
ROD PROJECTION AND GROUT THICKNESS SCHEDULE		
ANCHOR ROD DIAMETER	GROUT THICKNESS	MIN. ROD PROJECTION
1" OR LESS	1 1/2"	1 1/2"
1 1/8" TO 1 1/2"	2"	2"

2 COLUMN BASE PLATE TYPICAL DETAIL - WF
NOT TO SCALE



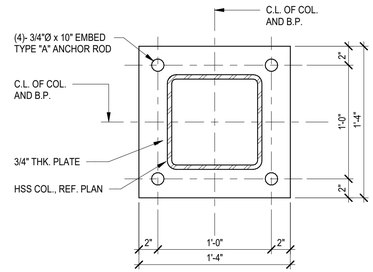
NOTES:
1. WELD TO BE MINIMUM SIZE REQUIRED BY A.I.S.C. FOR THICKNESS OF BASE PLATE USED.
2. REF. COLUMN BASE PLATE TYPICAL DETAIL - HSS FOR BASE PLATE ELEVATION.

6 BP-2
NOT TO SCALE



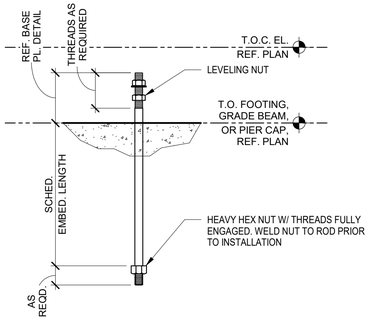
ROD PROJECTION AND GROUT THICKNESS SCHEDULE		
ANCHOR ROD DIAMETER	GROUT THICKNESS	MIN. ROD PROJECTION
1" OR LESS	1 1/2"	1 1/2"
1 1/8" TO 1 1/2"	2"	2"

3 COLUMN BASE PLATE TYPICAL DETAIL - HSS
NOT TO SCALE

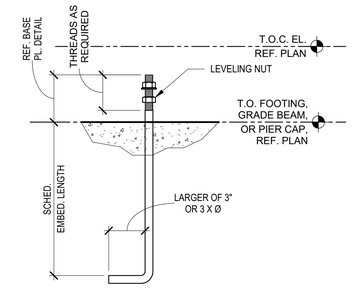


NOTES:
1. WELD TO BE MINIMUM SIZE REQUIRED BY A.I.S.C. FOR THICKNESS OF BASE PLATE USED.
2. REF. COLUMN BASE PLATE TYPICAL DETAIL - HSS FOR BASE PLATE ELEVATION.

7 BP-3
NOT TO SCALE



B TYPE "B" ROD



A TYPE "A" ROD

NOTES:
i. USE TYPE "A" ROD, TYPICAL U.N.O.
ii. USE TYPE "B" ROD AT ALL BRACE LOCATIONS AND OTHER LOCATIONS AT CONTRACTOR'S OPTION.
iii. HEADED BOLTS MAY BE USED IN LIEU OF TYPE "A" OR TYPE "B" RODS AT CONTRACTOR'S OPTION.

4 ANCHOR ROD TYPES
NOT TO SCALE

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No.	Date	Issue / Revision

CODY LAMBERT, State of Florida, Professional Engineer, License No. 100455

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WGI, INC.
3111 W. DR. MARTIN LUTHER KING JR. BLVD.
SUITE 375
TAMPA, FL 33607

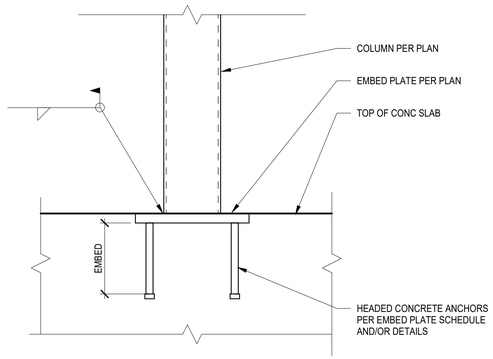
FLORIDA-ALABAMA TPO

ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

STEEL COLUMN BASE PLATE DETAILS

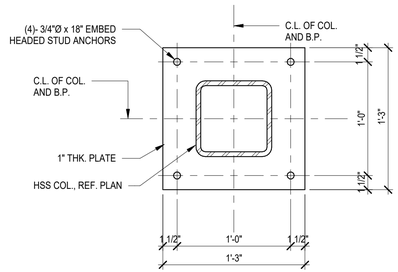
DWG S-557

SHEET



1 TYPICAL COLUMN EMBED PLATE ELEVATION

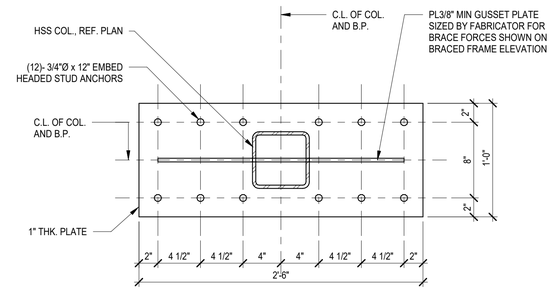
NOT TO SCALE



NOTES:
1. WELD TO BE MINIMUM SIZE REQUIRED BY A.I.S.C. FOR THICKNESS OF BASE PLATE USED.
2. REF. COLUMN EMBED PLATE TYPICAL ELEVATION DETAIL FOR ADD'L INFO.

2 EP-A

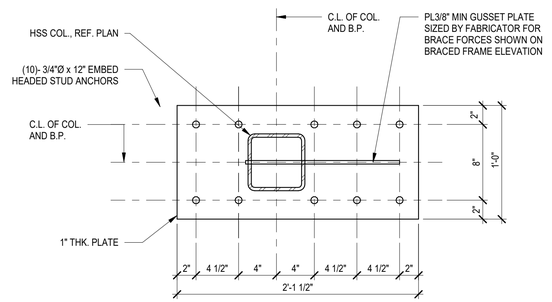
NOT TO SCALE



NOTES:
1. WELD TO BE MINIMUM SIZE REQUIRED BY A.I.S.C. FOR THICKNESS OF BASE PLATE USED.
2. REF. COLUMN EMBED PLATE TYPICAL ELEVATION DETAIL FOR ADD'L INFO.

3 EP-B

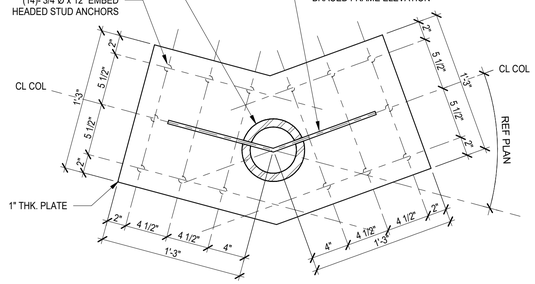
NOT TO SCALE



NOTES:
1. WELD TO BE MINIMUM SIZE REQUIRED BY A.I.S.C. FOR THICKNESS OF BASE PLATE USED.
2. REF. COLUMN EMBED PLATE TYPICAL ELEVATION DETAIL FOR ADD'L INFO.

4 EP-C

NOT TO SCALE



NOTES:
1. WELD TO BE MINIMUM SIZE REQUIRED BY A.I.S.C. FOR THICKNESS OF BASE PLATE USED.
2. REF. COLUMN EMBED PLATE TYPICAL ELEVATION DETAIL FOR ADD'L INFO.

5 EP-D

SCALE: 1 1/2" = 1'-0"

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3111 W. DR. MARTIN LUTHER KING JR. BLVD.
SUITE 375
TAMPA, FL 33607

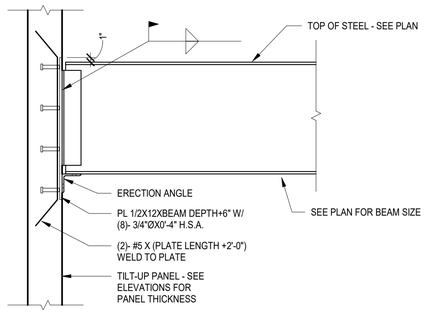
FLORIDA-ALABAMA TPO

ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

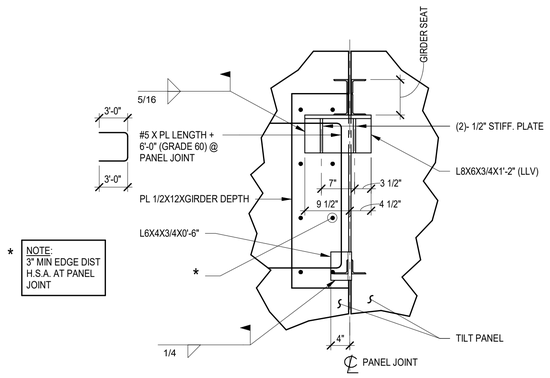
STEEL COLUMN EMBED DETAILS

DWG S-558

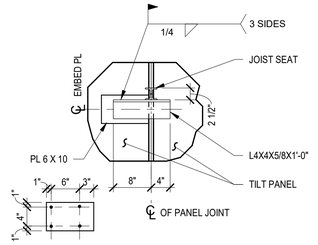
SHEET



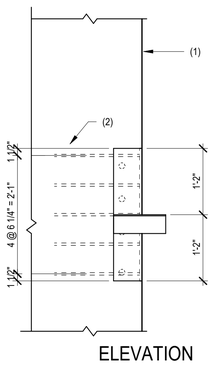
1 BEAM BEARING AT PANEL
NOT TO SCALE



2 CONNECTION AT PANEL JOINT
NOT TO SCALE

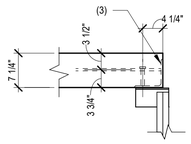


3 CONNECTION AT PANEL JOINT (JOIST BEARING)
NOT TO SCALE

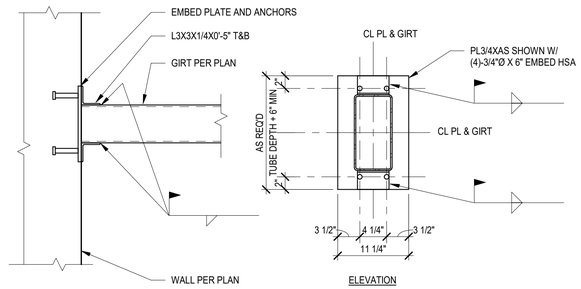


ELEVATION

1. END OF PANEL
2. T.O.S.
3. L6X6X3/8X2'-4" W/ (4) 3/4"ØX4" H.S.A.
& (5) 3/4"ØX2'-0" D.B.A.



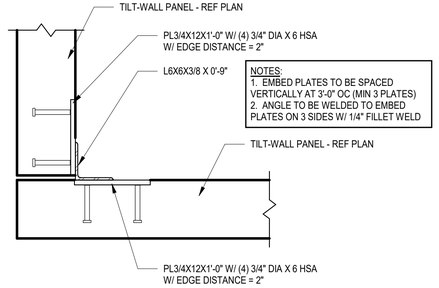
PLAN



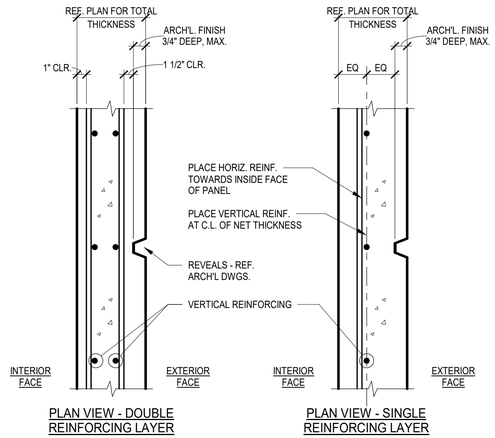
ELEVATION

4 JOIST TO PANEL EDGE
NOT TO SCALE

5 GIRT CONNECTION AT PANEL
NOT TO SCALE



6 PARTIAL PLAN - CORNER TILT WALL PANEL
NOT TO SCALE



7 TYPICAL TILT-UP PANEL THICKNESS & REINFORCING PLACEMENT DETAIL
NOT TO SCALE

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No.	Date	Issue / Revision

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FL, 100455

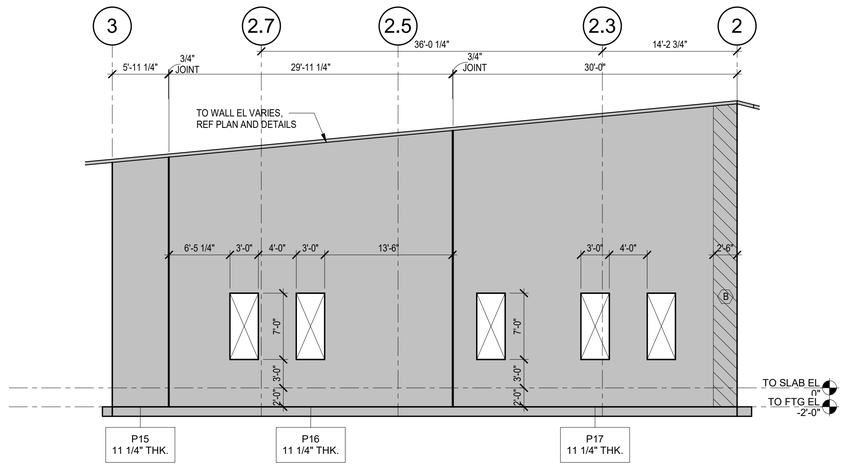
WGI, INC.
3111 W. DR. MARTIN LUTHER KING JR. BLVD.
SUITE 375
TAMPA, FL 33607

FLORIDA-ALABAMA TPO

ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

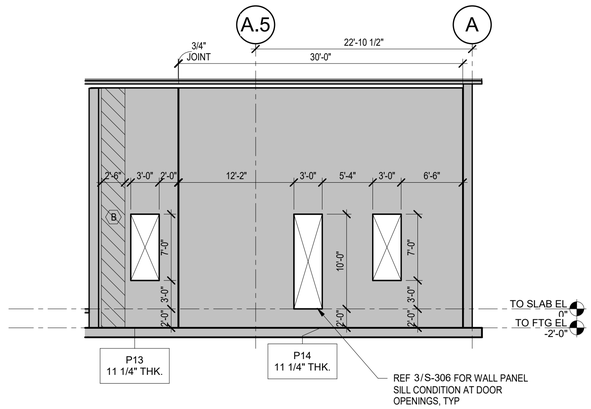
TYPICAL TILT WALL DETAILS

DWG
S-600
SHEET



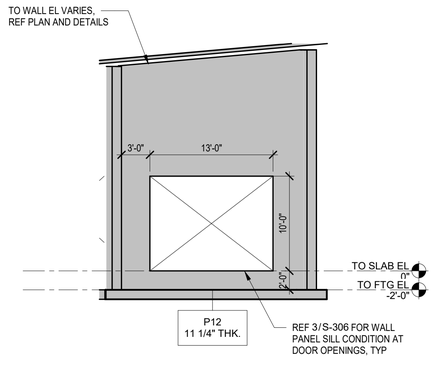
1 TILT-UP WALL ELEVATION

SCALE: 1/8" = 1'-0"



2 TILT-UP WALL ELEVATION

SCALE: 1/8" = 1'-0"

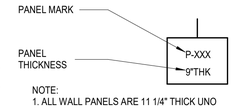


3 TILT-UP WALL ELEVATION

SCALE: 1/8" = 1'-0"

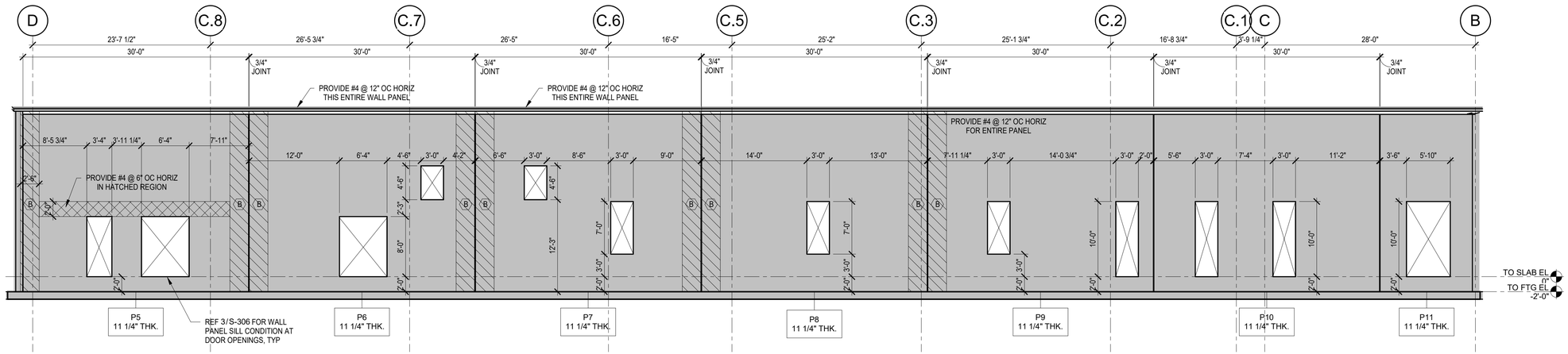
TILT WALL PANEL NOTES:

1. REF ARCH DRAWINGS AND SPECIFICATIONS FOR FINISHES, CHAMFERS AND REVEALS
2. REF. MECH. DRAWINGS FOR ALL MECH. OPENINGS AND SLEEVES. PROVIDE (2)- #5 BARS ON ALL SIDES OF OPENING U.N.O.
3. REF. ARCH AND STRUCTURAL DRAWINGS FOR ITEMS TO BE CAST IN PANELS
4. PANEL LIFTING IS THE RESPONSIBILITY OF THE CONTRACTOR. ADDITIONAL REINFORCING AND STRONG BACK FOR PANEL ERECTION SHALL BE SUPPLIED AS REQ'D. CONTRACTOR SHALL SUPPLY LIFTING INSERT SHOP DRAWINGS DETAILING EXACT LOCATION AND BRACING INSERT LOCATIONS.
5. SOIL FILL ADJACENT TO EXTERIOR TILT-WALL PANELS SHALL BE HAND COMPACTED TO THE REQUIRED DENSITIES.
6. PROVIDE 3/4" JOINT (VERIFY) BETWEEN ALL PANELS.
7. REFER TO S-620 FOR ADD'L WALL REINF INFORMATION.
8. PANELS SHOWN IN ELEVATIONS ARE VIEWED FROM THE BUILDING EXTERIOR, TYP.
9. REFER TO S-621 FOR TYPICAL WALL PANEL REINFORCEMENT ELEVATION.
10. REFER TO PLAN AND SECTIONS FOR LOCATING TOP OF WALL ELEVATION, TYP UNO.



NOTE:
1. ALL WALL PANELS ARE 11 1/4" THICK UNO

TYPICAL TILT-UP PANEL CALLOUT



4 TILT-UP WALL ELEVATION

SCALE: 1/8" = 1'-0"

No.	Date	Issue / Revision

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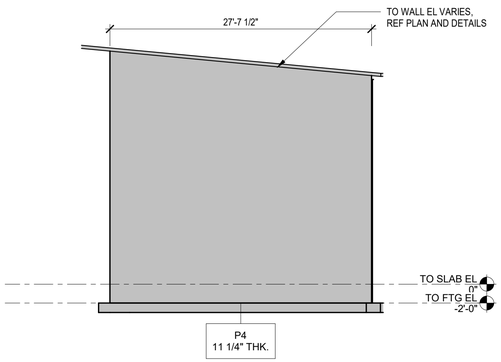
SUITE 375
TAMPA, FL 33607

FLORIDA-ALABAMA TPO		
ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

TILT-WALL ELEVATION

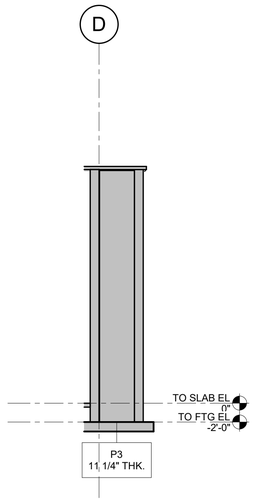
DWG S-610

SHEET



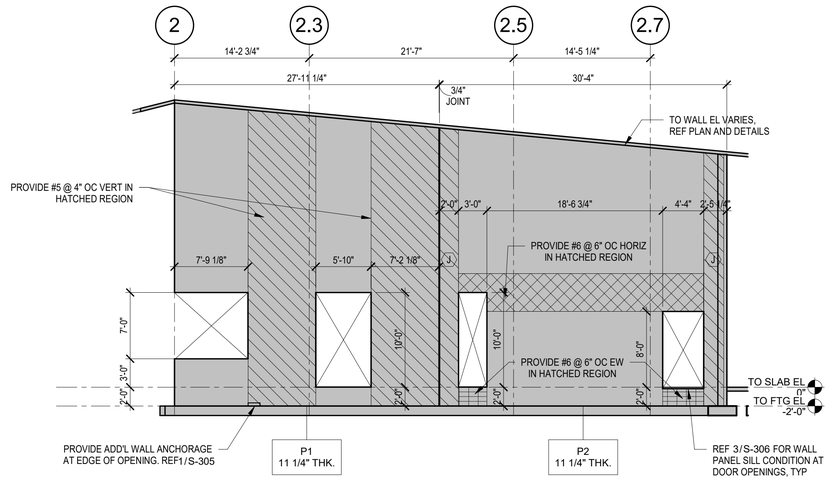
1 TILT-UP WALL ELEVATION

SCALE: 1/8" = 1'-0"



2 TILT-UP WALL ELEVATION

SCALE: 1/8" = 1'-0"

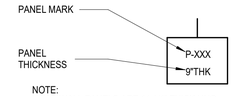


3 TILT-UP WALL ELEVATION

SCALE: 1/8" = 1'-0"

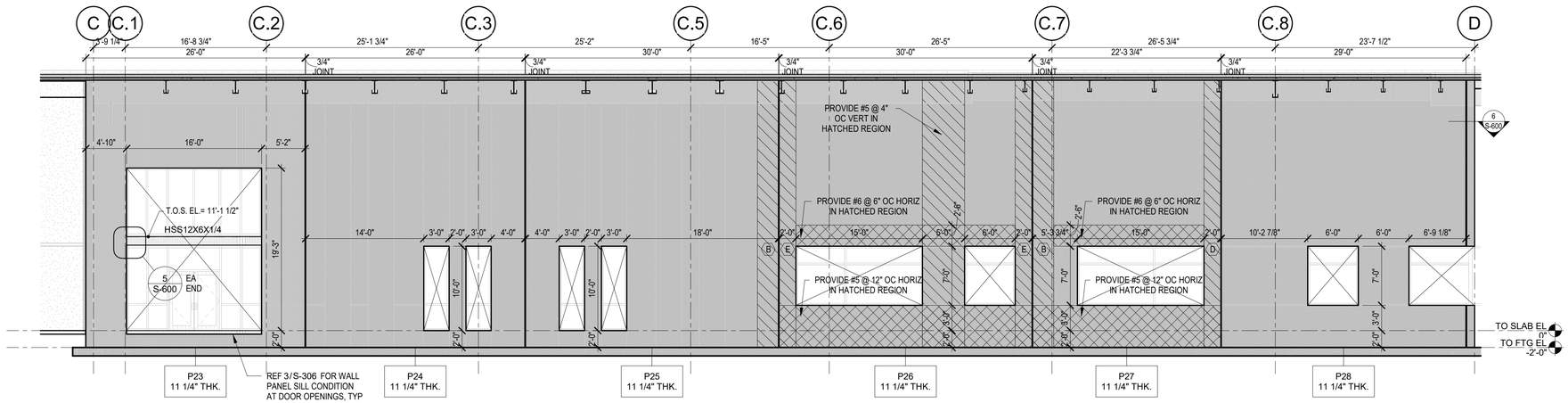
TILT WALL PANEL NOTES:

1. REF ARCH DRAWINGS AND SPECIFICATIONS FOR FINISHES, CHAMFERS AND REVEALS
2. REF. MECH. DRAWINGS FOR ALL MECH. OPENINGS AND SLEEVES. PROVIDE (2)- #5 BARS ON ALL SIDES OF OPENING U.N.O.
3. REF. ARCH AND STRUCTURAL DRAWINGS FOR ITEMS TO BE CAST IN PANELS
4. PANEL LIFTING IS THE RESPONSIBILITY OF THE CONTRACTOR. ADDITIONAL REINFORCING AND STRONG BACK FOR PANEL ERECTION SHALL BE SUPPLIED AS REQ'D. CONTRACTOR SHALL SUPPLY LIFTING INSERT SHOP DRAWINGS DETAILING EXACT LOCATION AND BRACING INSERT LOCATIONS.
5. SOIL FILL ADJACENT TO EXTERIOR TILT-WALL PANELS SHALL BE HAND COMPACTED TO THE REQUIRED DENSITIES.
6. PROVIDE 3/4" JOINT (VERIFY) BETWEEN ALL PANELS.
7. REFER TO S-620 FOR ADD'L WALL REINF INFORMATION.
8. PANELS SHOWN IN ELEVATIONS ARE VIEWED FROM THE BUILDING EXTERIOR, TYP.
9. REFER TO S-621 FOR TYPICAL WALL PANEL REINFORCEMENT ELEVATION.
10. REFER TO PLAN AND SECTIONS FOR LOCATING TOP OF WALL ELEVATION, TYP UNO.



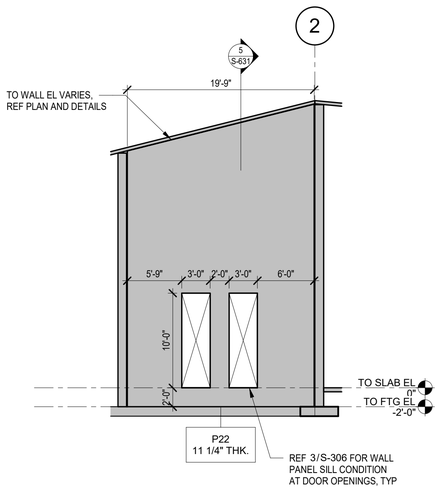
NOTE:
1. ALL WALL PANELS ARE 11 1/4" THICK UNO

TYPICAL TILT-UP PANEL CALLOUT



4 TILT-UP WALL ELEVATION

SCALE: 1/8" = 1'-0"



5 TILT-UP WALL ELEVATION

SCALE: 1/8" = 1'-0"

No.	Date	Issue / Revision

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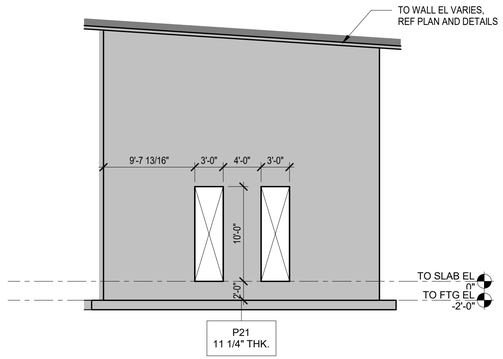
TAMPA, FL 33607

FLORIDA-ALABAMA TPO

ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

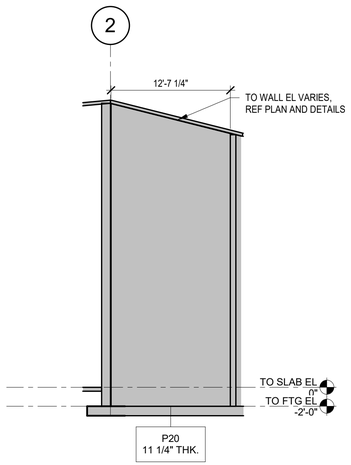
TILT-WALL ELEVATION

DWG
S-611
SHEET



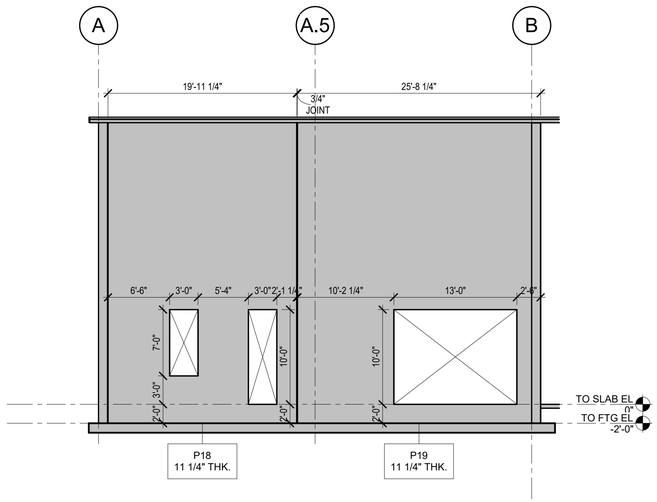
1 TILT-UP WALL ELEVATION

SCALE: 1/8" = 1'-0"



2 TILT-UP WALL ELEVATION

SCALE: 1/8" = 1'-0"



3 TILT-UP WALL ELEVATION

SCALE: 1/8" = 1'-0"

TILT WALL PANEL NOTES:

1. REF ARCH DRAWINGS AND SPECIFICATIONS FOR FINISHES, CHAMFERS AND REVEALS
2. REF. MECH. DRAWINGS FOR ALL MECH. OPENINGS AND SLEEVES. PROVIDE (2)- #5 BARS ON ALL SIDES OF OPENING U.N.O.
3. REF. ARCH AND STRUCTURAL DRAWINGS FOR ITEMS TO BE CAST IN PANELS
4. PANEL LIFTING IS THE RESPONSIBILITY OF THE CONTRACTOR. ADDITIONAL REINFORCING AND STRONG BACK FOR PANEL ERECTION SHALL BE SUPPLIED AS REQ'D. CONTRACTOR SHALL SUPPLY LIFTING INSERT SHOP DRAWINGS DETAILING EXACT LOCATION AND BRACING INSERT LOCATIONS.
5. SOIL FILL ADJACENT TO EXTERIOR TILT-WALL PANELS SHALL BE HAND COMPACTED TO THE REQUIRED DENSITIES.
6. PROVIDE 3/4" JOINT (VERIFY) BETWEEN ALL PANELS.
7. REFER TO S-620 FOR ADD'L WALL REINF INFORMATION.
8. PANELS SHOWN IN ELEVATIONS ARE VIEWED FROM THE BUILDING EXTERIOR, TYP.
9. REFER TO S-621 FOR TYPICAL WALL PANEL REINFORCEMENT ELEVATION.
10. REFER TO PLAN AND SECTIONS FOR LOCATING TOP OF WALL ELEVATION, TYP UNO.

NOTE:
1. ALL WALL PANELS ARE 11 1/4" THICK UNO

TYPICAL TILT-UP PANEL CALLOUT

No.	Date	Issue / Revision

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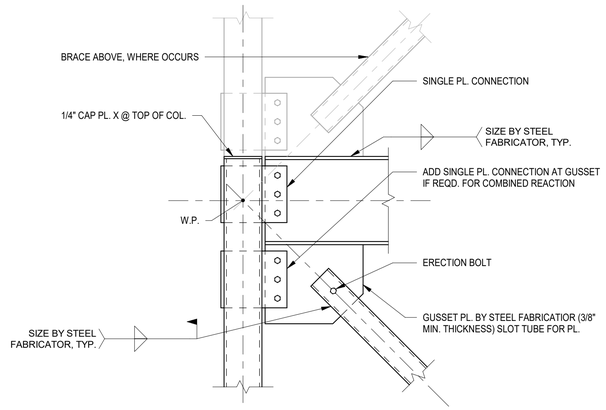
TAMPA, FL 33607

FLORIDA-ALABAMA TPO

ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

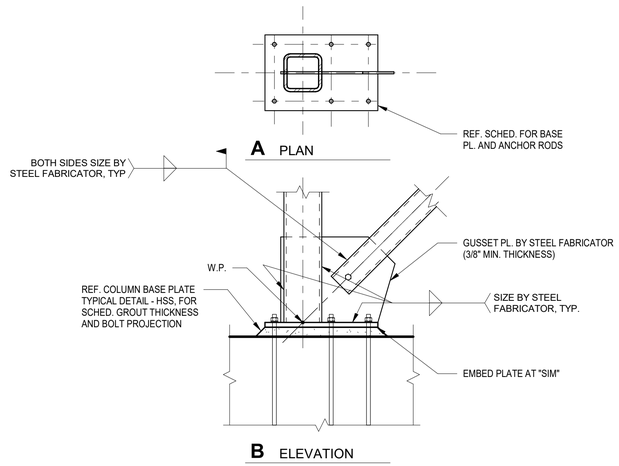
TILT-WALL ELEVATION

DWG
S-612
SHEET



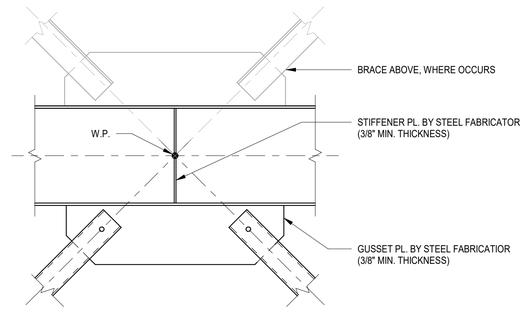
1 WIDE FLANGE BEAM AND HSS BRACE CONNECTION TO TUBE COLUMN TYPICAL DETAIL

NOT TO SCALE



2 HSS BRACE CONNECTION TO TUBE COLUMN AND BASE PLATE TYPICAL DETAIL

NOT TO SCALE

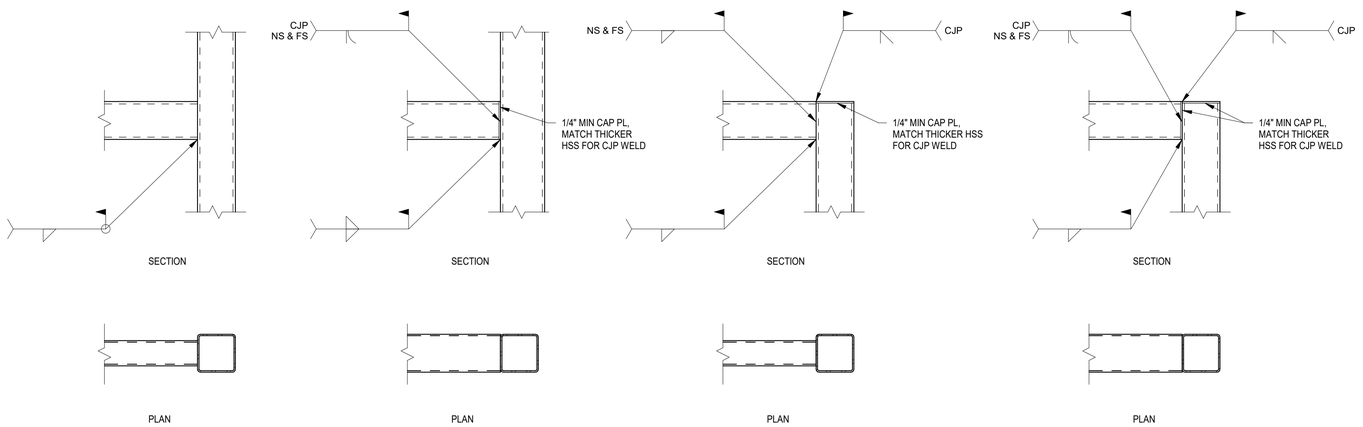


3 HSS BRACE CONNECTION TO BEAM TYPICAL DETAIL

NOT TO SCALE

BRACE DESIGN NOTES:

1. DESIGN CONNECTIONS FOR FORCES SHOWN ON BRACE ELEVATIONS.
2. IF NO FORCES ARE SHOWN ON BRACE ELEVATIONS, DESIGN FOR THE FULL TENSILE CAPACITY OF THE BRACES ACCORDING TO THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION MANUAL. REFER TO GENERAL NOTES FOR MANUAL EDITION.
3. SIZE WELDS FOR FORCES AND ECCENTRICITY OF C.G. WELD AND C.G. FORCE.
4. AT BEAM TO COLUMN CONNECTIONS, SIZE ATTACHMENT TO COLUMN FOR COMBINED GRAVITY LOAD SHEAR PLUS VERTICAL SHEAR COMPONENT DUE TO LATERAL LOAD.
5. CHECK FOR WHITMORE SECTION BUCKLING AND BLOCK SHEAR RUPTURE OF GUSSET PLATES.
6. CALCULATIONS SHALL BE PERFORMED AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER AND SHALL BE SUBMITTED TO THE ARCHITECT PER SPECIFICATIONS.
7. DESIGN FORCES NOTED AS H=. K ARE FACTORED FORCES IN AXIAL TENSION (T) OR COMPRESSION (C). WHERE LOADS "H" ARE SPECIFIED, THE MAGNITUDE OF THE LOAD SHALL BE USED FOR BOTH TENSION AND COMPRESSION.



4 HSS TO HSS WELD CONNECTIONS, TYP

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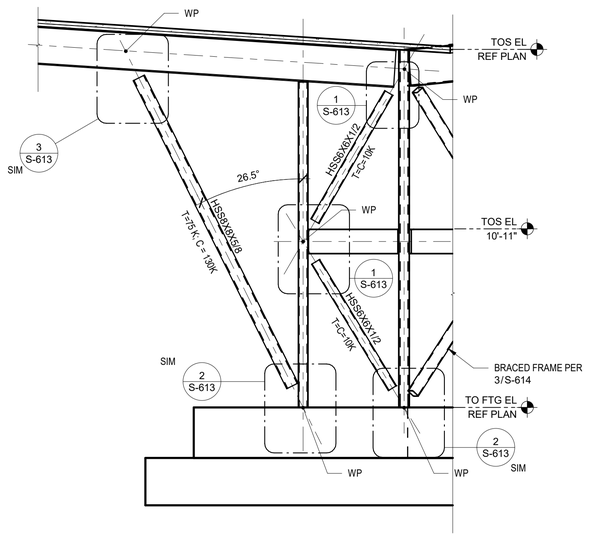
FLORIDA-ALABAMA TPO

ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

TYPICAL BRACE DETAILS

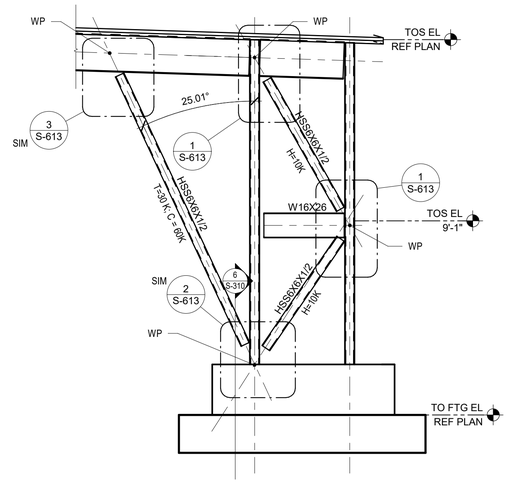
DWG S-613

SHEET



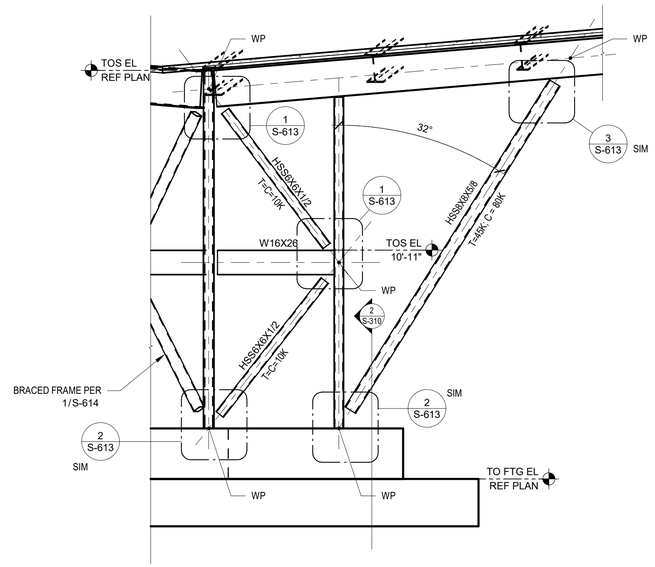
1 STEEL BRACE FRAME ELEVATION

SCALE: 1/4" = 1'-0"



2 STEEL BRACE FRAME ELEVATION

SCALE: 1/4" = 1'-0"



3 STEEL BRACE FRAME ELEVATION

SCALE: 1/4" = 1'-0"

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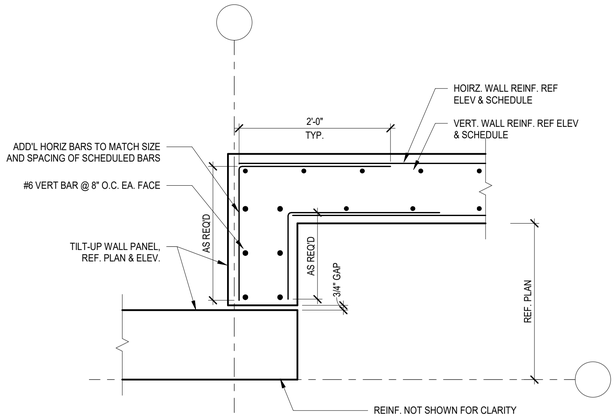
FLORIDA-ALABAMA TPO

ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

BRACED FRAME ELEVATIONS

DWG
S-614
SHEET



1 PANEL LEG REINF.

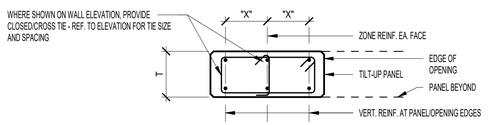
NOT TO SCALE

VERTICAL WALL REINFORCING SCHEDULE		
ZONE CALLOUT	BAR SIZE	QUANTITY - EA. FACE
A	#5	3
B	#5	4
C	#5	5
D	#5	6
E	#5	7
F	#5	8
G	#5	9
H	#5	10
J	#8	6

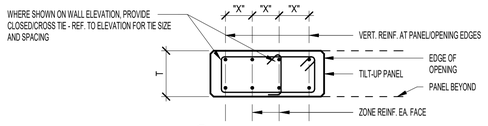
NOTES:
1. QUANTITY OF BARS ARE TO BE EQUALLY SPACED WITHIN THE MARKED ZONES NOTED ON ELEVATIONS
2. FOR PANEL ELEVATIONS REFER TO PLAN
3. NOTED REINFORCEMENT REPLACES TYPICAL VERTICAL BARS NOTED IN TYP. PANEL ELEVATIONS, NOT IN ADDITION TO.
4. REF. TO DETAIL 3/S-620 SHEET FOR REINF. LAYOUT

2 VERTICAL WALL REINF. SCHEDULE

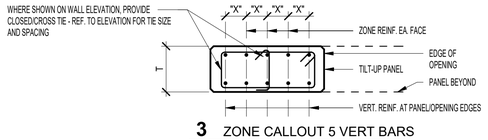
NOT TO SCALE



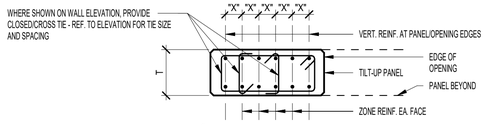
1 ZONE CALLOUT 3 VERT BARS



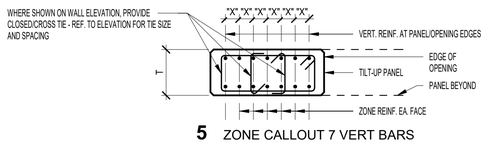
2 ZONE CALLOUT 4 VERT BARS



3 ZONE CALLOUT 5 VERT BARS



4 ZONE CALLOUT 6 VERT BARS

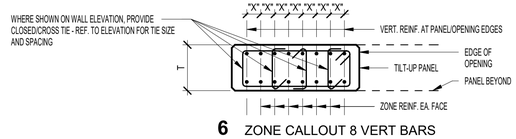


5 ZONE CALLOUT 7 VERT BARS

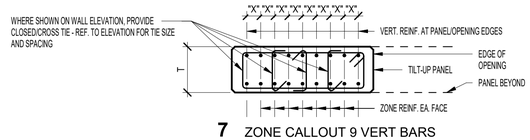
NOTES:
1. T = PANEL THICKNESS - REF. TO ELEVATION
2. REF. TO 1/S-621 THIS SHEET FOR REQ'D VERT REINF IN ZONES
3. "X" DENOTES VERT. REINF. SPACING
4. CROSS-TIES ARE REQ'D AT EVERY VERT. IF "X" IS GREATER THAN 6"
5. CROSS-TIES SHALL HAVE 135 DEG. HOOK AT ONE END AND STD. 90 DEG. HOOK AT OTHER END
6. CONSECUTIVE CROSS-TIES SHALL BE PLACED SO THAT HOOKS ARE ALTERNATING
7. TIES AND CROSS-TIES SHALL BE #4 @ 12" OC UNO

3 TILT-UP PANEL VERT REINFORCMENT BOUNDARY ZONES

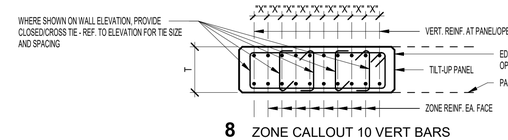
NOT TO SCALE



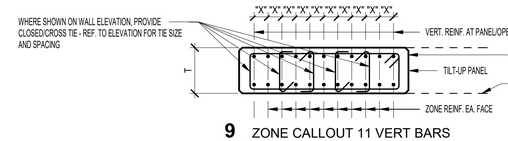
6 ZONE CALLOUT 8 VERT BARS



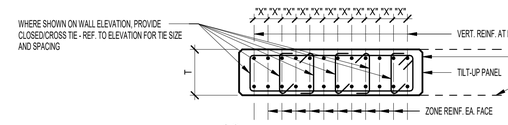
7 ZONE CALLOUT 9 VERT BARS



8 ZONE CALLOUT 10 VERT BARS



9 ZONE CALLOUT 11 VERT BARS



10 ZONE CALLOUT 12 VERT BARS

11 ZONE CALLOUT 13 VERT BARS

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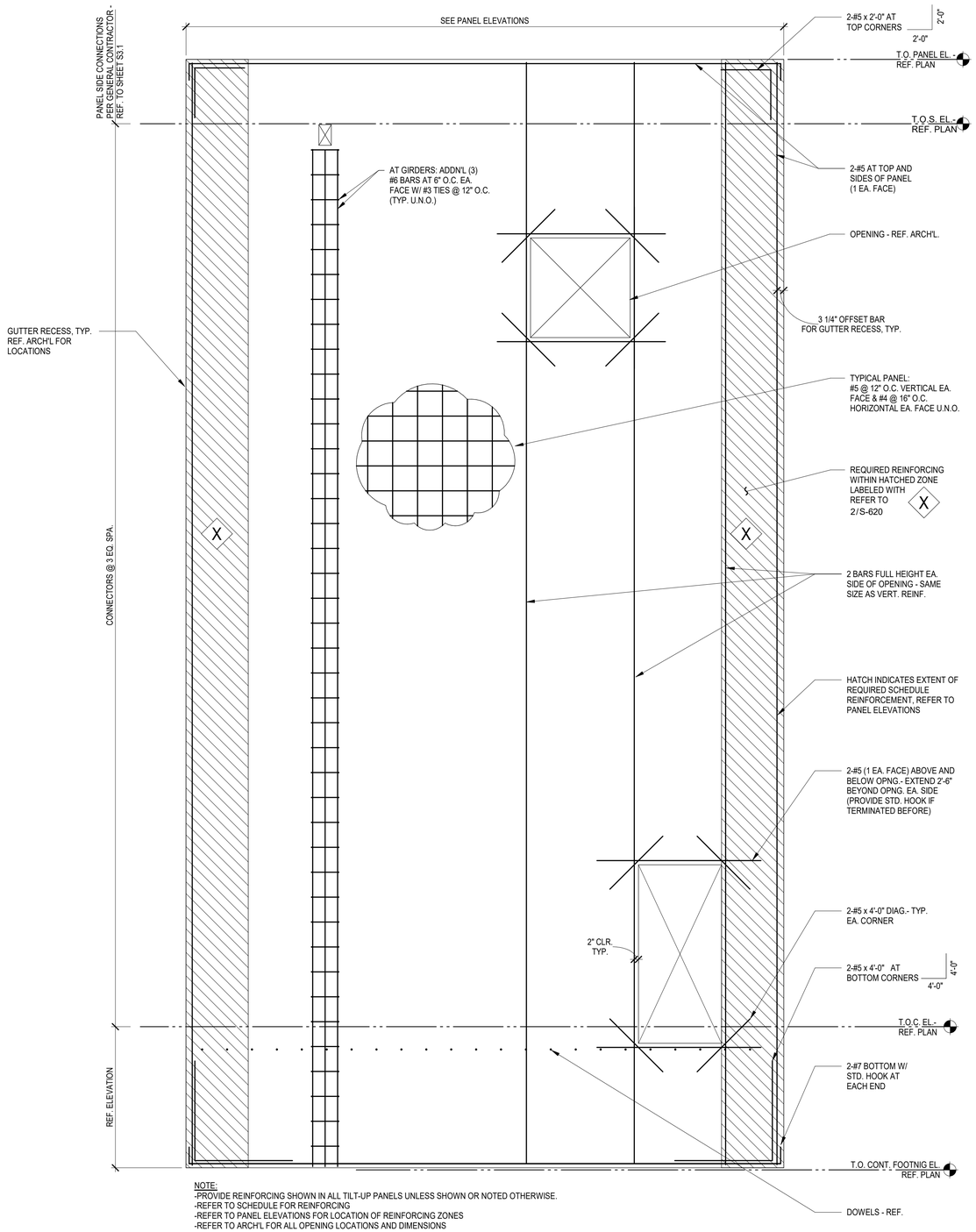
FLORIDA-ALABAMA TPO

ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

TYPICAL TILT REINF. WALL ELEVATIONS

DWG
S-620
SHEET



NOTE:
-PROVIDE REINFORCING SHOWN IN ALL TILT-UP PANELS UNLESS SHOWN OR NOTED OTHERWISE.
-REFER TO SCHEDULE FOR REINFORCING
-REFER TO PANEL ELEVATIONS FOR LOCATION OF REINFORCING ZONES
-REFER TO ARCH'L FOR ALL OPENING LOCATIONS AND DIMENSIONS

1 TYPICAL TILT-UP WALL PANEL ELEVATION

NOT TO SCALE

No.	Date	Issue / Revision

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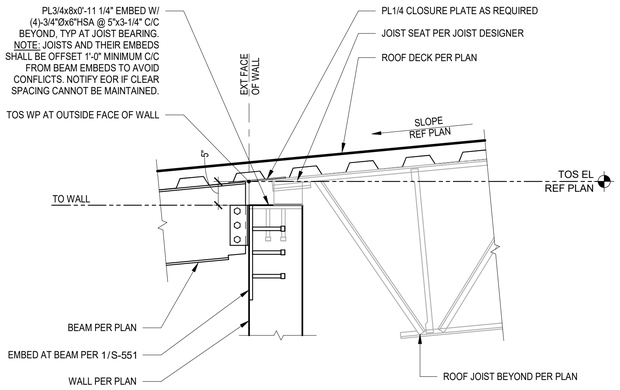
FLORIDA-ALABAMA TPO

ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

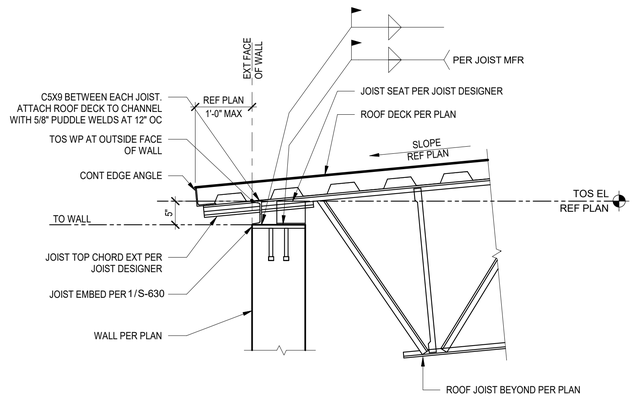
TYPICAL TILT REINF. WALL ELEVATIONS

DWG
S-621
SHEET



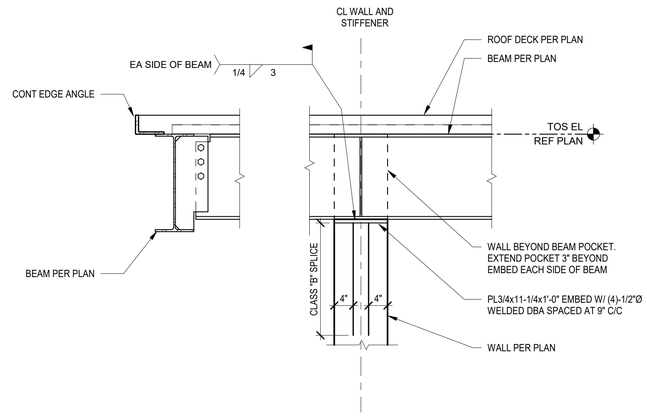
1 DETAIL - ROOF FRAMING ACROSS EXT WALL

SCALE: 3/4" = 1'-0"



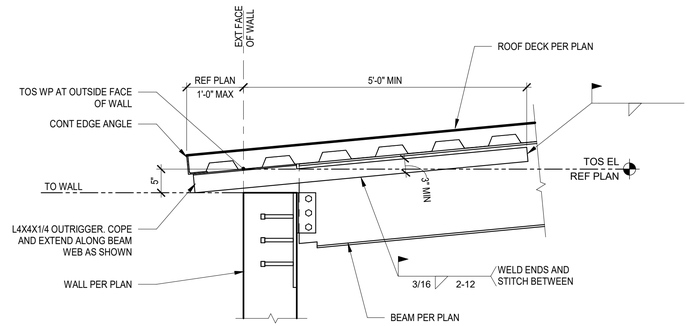
2 DETAIL - ROOF AT EXT WALL

SCALE: 3/4" = 1'-0"



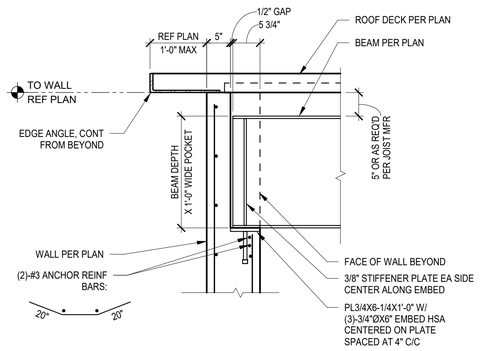
3 DETAIL- GIRDER POCKET AT CANTILEVER

SCALE: 3/4" = 1'-0"



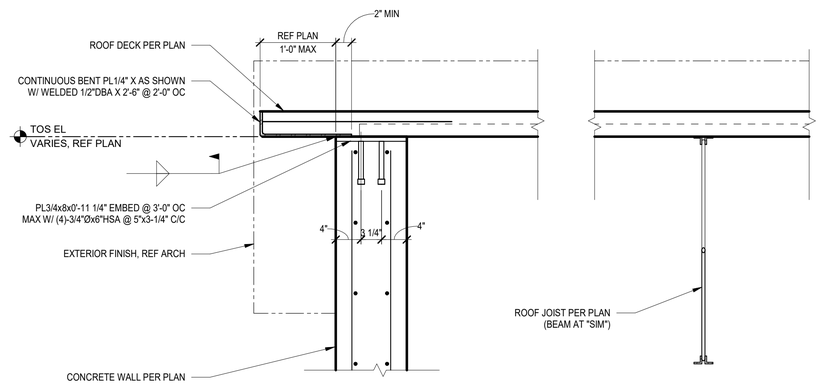
4 DETAIL - BEAM BEARING AT WALL

SCALE: 3/4" = 1'-0"



5 SECTION AT GIRDER POCKET

SCALE: 3/4" = 1'-0"



6 SECTION

SCALE: 1" = 1'-0"

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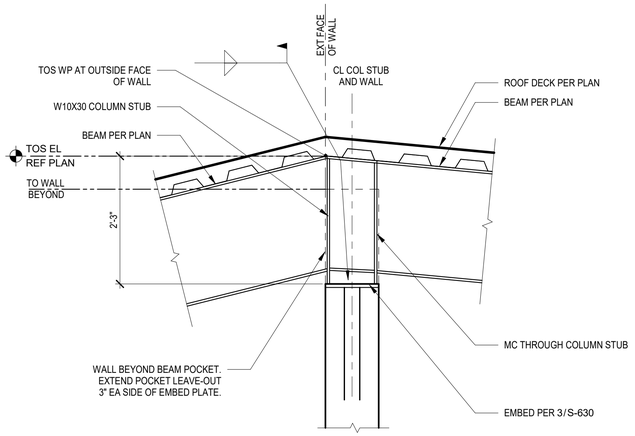
FLORIDA-ALABAMA TPO

ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

ROOF FRAMING DETAILS

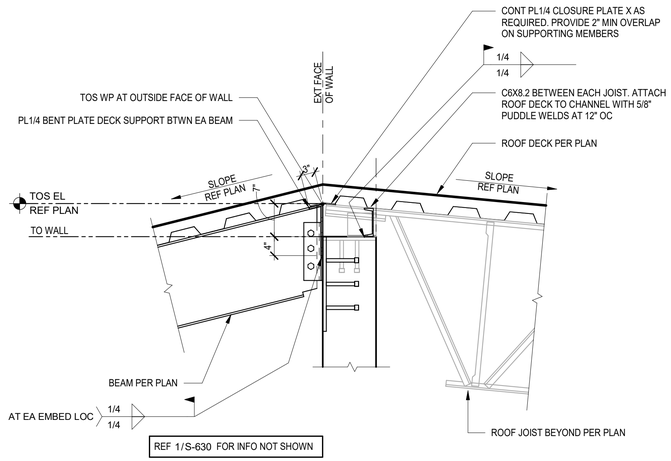
DWG S-630

SHEET



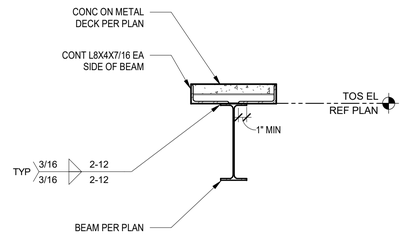
1 DETAIL AT KINKED CONTINUOUS BEAM

SCALE: 3/4" = 1'-0"



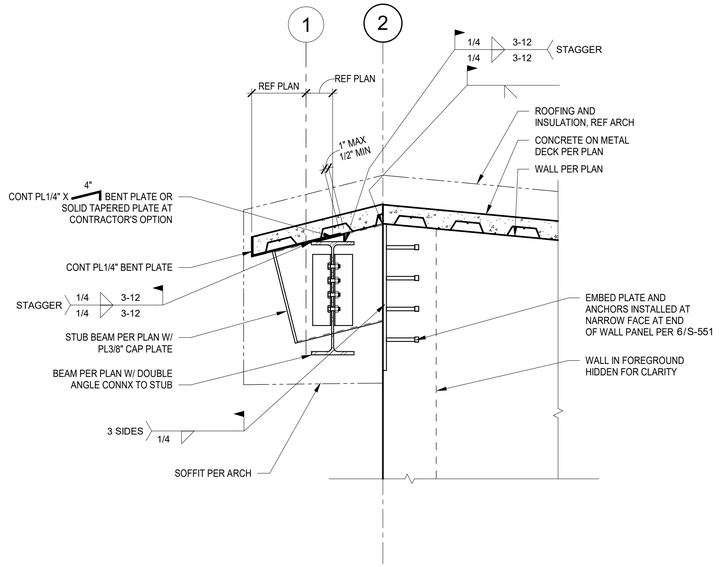
2 DETAIL - ROOF FRAMING EA SIDE OF WALL AT HIGH SIDE

SCALE: 3/4" = 1'-0"



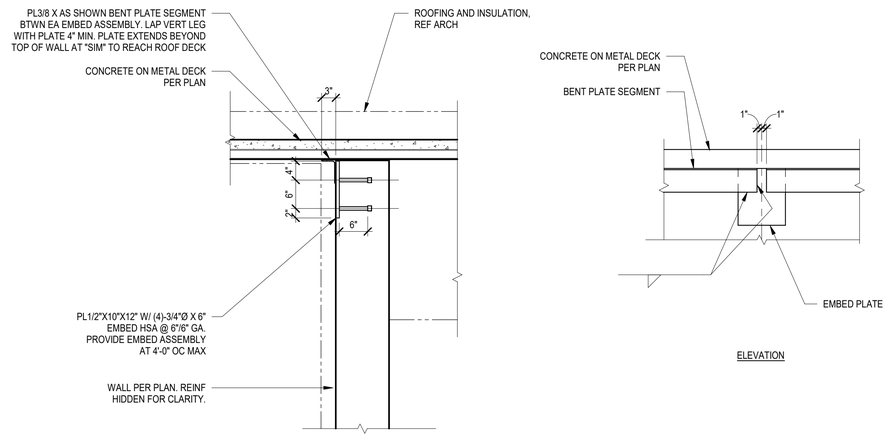
3 SECTION

SCALE: 3/4" = 1'-0"



4 SECTION

SCALE: 3/4" = 1'-0"



5 DETAIL

SCALE: 3/4" = 1'-0"

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FLORIDA-ALABAMA TPO

ROAD	COUNTY	FINANCIAL PROJECT
NORTH W STREET	ESCAMBIA	451524-1-38-01

ROOF FRAMING DETAILS

DWG
S-631
SHEET